ENGINEERING INVESTIGATION & ANALYSIS
GEOTECHNICAL & STRUCTURAL
ASSESSMENT REPORT

30 CARLOS DRIVE
FAIRFIELD, NEW JERSEY 08203

MATRIXNEWORLD
Engineering Progress

Prepared for:

State of New Jersey
Department of Community Affairs
PO Box 800
Trenton, NJ 08625-0800

Prepared by:

Matrix New World Engineering, Land Surveying
and Landscape Architecture, PC
26 Columbia Turnpike
Florham Park, New Jersey 07932

Matrix No. 20-1052

August 2021

Michael J. Soltys, P.E.
Director of Structural & Geotechnical Engineering

TABLE OF CONTENTS

FIGURES

APPENDICES

1.0 PROJECT BACKGROUND

The State of New Jersey Department of Community Affairs (DCA), Division of Disaster Recovery and Mitigation, anticipates receiving approval for grant funding through FEMA's Flood Mitigation Assistance (FMA) appropriation. This funding is provided through FMA to states and local communities to reduce or eliminate flood risk due to repetitive flood damage to buildings insured by the National Flood Insurance Program (NFIP). The DCA intends to use the funding for the State's Mitigation Assistance Program (MAP) to elevate residential properties located in a floodplain in the Township of Fairfield. The properties are to be elevated at least 3 feet above the base flood elevation (BFE). The DCA hosted a town hall meeting for homeowners in Fairfield, focused on homeowners with properties that experience Repetitive Losses or Severe Repetitive Losses.

In preparation of procuring a Design-Build firm to conduct the effort, the DCA has contracted Matrix New World Engineering, Land Surveying and Landscape Architecture, P.C. (Matrix) to conduct a geotechnical analysis, preliminary structural analysis, and elevation certificate for residences anticipated to be included in the program. It is understood that this document will serve as the basis for the development of a Request for Proposal (RFP) to procure Design-Build firms to do final structural design and perform the elevation of the properties.

2.0 PROJECT SCOPE

Matrix has completed a geotechnical and structural assessment and elevation certificate to evaluate the viability of elevating the residential building located at 30 Carlos Drive in Fairfield, New Jersey (Site). Matrix provided geotechnical and structural engineering and land surveying services as a consultant to the DCA. The project location is shown on the attached Site Location Map (Figure 1).

The purpose of the engineering study was to compile comprehensive data regarding the existing building's foundations and overall structural composition and condition at the Site. The information obtained will be further utilized to determine the feasibility and proposed design of raising the existing residence 3 feet above the base flood elevation (BFE) as determined by FEMA. A team of Matrix engineers and surveyors performed the evaluation, consisting of a geotechnical soil inspection, test pits to reveal the existing building foundations, an interior inspection of the building's visible foundation walls and frame, and topographic surveying for the development of a flood elevation certificate. One test pit (TP-1) was completed to a depth of 73 inches below the ground surface (bgs) and 2 geotechnical borings (B-1 and B-2) were completed to a depth of 27 feet bgs (see Figure 2).

Matrix's geotechnical characterization of the property is based on an engineering evaluation of the subsurface conditions as indicated by the field exploration data and geotechnical laboratory test results on representative soil samples.

3.0 SITE LOCATION & PROJECT DESCRIPTION

The project site is located at 30 Carlos Drive in Fairfield, New Jersey. The property consists of a two-story timber-framed raised ranch with an approximately 1,820 square foot footprint. The house is situated atop concrete masonry unit (CMU) foundation walls on cast-in-place concrete foundations. The substructure of the house is comprised of a finished basement area which encompasses the full two-story house. The house also contains an attached garage area adjacent to the north wall of the building. The timber frame of the residential structure is covered with a brick façade throughout its first-floor exterior and a vinyl siding on the second-floor exterior walls. The property also contains a timber-framed painted timber deck in the rear of the house that spans the full width of the building.

To assist with the geotechnical and structural evaluation, test pits and geotechnical borings were advanced in areas around the residence to obtain information regarding the soil's structural properties and building's existing foundation. The test pit and 2 borings were located to provide the most useful information about the subsurface conditions. Refer to Figure 2 of this report for a map of the test pit and boring locations.

4.0 GEOLOGIC SETTING

According to the USDA Soil Survey of Essex County, the site is situated atop Horseneck – Urban land. The subsurface composition is typically sand and loamy sands from 8 to 60 inches bgs.

According to the 2014 Bedrock Geologic Map of New Jersey, the Site is underlain by the Sedimentary and Bedded Volcanic Rocks Towaco Formation. Specifically, the subsurface consists of micaceous, reddish-brown sandstone, siltstone and silty mudstone in upward-fining sequences. The Bedrock Geologic Map is shown in Figure 3.

From the Surficial Geologic Map of Northern New Jersey, compiled by and edited by Byron D. Stone, Scott D. Stanford, and Ron W. White in 2002, the natural surface material (beyond fill) is suggested to be in the Pine Brook terrace deposit, which contains sand and gravel, moderately to poorly sorted. The Surficial Geology map is shown in Figure 4.

The documented site conditions presented above are consistent with the findings from the subsurface investigation, in which Sand was encountered followed by a layer of Silt and or Clay. Groundwater was encountered in the borings at approximately 4.2 to 4.3 feet bgs. Bedrock was not encountered during this subsurface program.

5.0 SUBSURFACE FIELD PROGRAM

The subsurface investigation was completed by generally accepted practices in the Geotechnical Engineering field and consisted of the advancement of 1 test pit and 2 Standard Penetration Test (SPT) borings using mud rotary drilling methods. Matrix retained Boring Brothers, Inc., located in Egg Harbor Township, NJ, to complete the subsurface field program.

A Matrix Geotechnical Engineer provided full-time drilling oversight, soil logging, and sample collection. Matrix prepared the field test pit and boring logs, which included sample depths, SPT-N blow counts, soil recovery, and soil descriptions based on the Burmister Soil Classification System followed by the Unified Soil Classification System (USCS) letter symbol. Test pit and soil boring logs are provided in Appendix A. Classification tables and charts used to determine the soil attributes are included in Appendix B.

Upon the completion of the field program, representative samples were subjected to geotechnical laboratory analyses. Laboratory results aided in soil classification and assessing the relevant engineering properties of the stratigraphic layers which were used in developing the revised geotechnical parameters outlined herein. Geotechnical laboratory reports are included in Appendix C.

5.1 Test Pits

On May 13, 2021, Boring Brothers completed a foundation survey which included 1 test pit (TP-1) to a depth of 73 inches below the ground surface. The test pit was dug using a Bobcat E55 and shovel to prevent any damage to the existing building foundations. The exterior edge of the building footing was exposed at both locations to accurately measure the structure's dimensions, as well as to analyze the conditions of the concrete foundation.

The Matrix Geotechnical Engineer also observed the subsurface soil conditions encountered within the test pit, noting the type and composition of the soils surrounding and beneath the existing footing. The test pit was backfilled with the original soils upon completion of the test pit log. No test pit samples were collected at the site for further analysis.

5.2 SPT Borings

On May 12, 2021, Boring Brothers advanced 2 geotechnical borings with a Mobile CME 55 track-mounted drill rig using mud rotary drilling techniques.

Split spoon (SS) samples were collected in accordance with *ASTM D-1586*, *Standard Method for Penetration Test and Split-Barrel Sampling of Soils*. A standard 2-inch outer diameter split spoon, two feet in length, was used to collect the soil samples. An automatic 140-pound hammer having a 30-inch drop was used to drive the split spoon sampler. As a part of boring observation, the SPT blow counts were recorded for the 0- to 6-inch interval, the 6- to 12-inch interval, the 12- to 18-inch interval and the 18- to 24-inch interval. The SPT N-values for design purposes are reported as the sum of the SPT N values observed for the above referenced 6- to 12-inch interval and the 12- to 18-inch interval that the split spoon sampler was driven.

The Matrix Geotechnical Engineer observed the split spoon samples and collected representative samples in sealed containers for further examination. All borings were continuously sampled to 12 feet bgs and at every subsequent 5-foot interval thereafter. The 2 borings were each advanced to a depth of 27 feet bgs. The borings were backfilled with soil cuttings and bentonite hole plug (if necessary) upon completion of the borehole.

5.3 Laboratory Testing

In addition to the field investigation, a laboratory testing program was conducted to determine additional pertinent engineering characteristics of representative samples of on-site soils. The laboratory testing program was performed in general accordance with applicable ASTM standard test methods and included physical/textural testing of representative samples of various strata.

Upon review of the boring logs, Matrix selected representative samples for laboratory testing. Laboratory testing of selected samples was completed by TerraSense, LLC, located in Totowa, New Jersey. The following table presents a summary of the testing program.

The results of the laboratory testing program were utilized to assist in developing geotechnical design parameters and recommendations, and are provided in Appendix C.

Table 5.3-1: Laboratory Testing Program

Test	Testing Procedure	Quantity Performed	Sample Locations and Depth Intervals
Water Content	ASTM D2216	4	B-1: 15-17', 25-27' B-2: 4-6', 20-22'
Sieve Analysis	ASTM D422	1	B-2: 4-6'
Atterberg Limits	ASTM D4318	2	B-1: 15-17', 25-27'
Combined Sieve & Hydrometer	ASTM D422	1	B-2: 20-22'

6.0 SUBSURFACE CONDITIONS

The subsurface conditions beneath the site can be characterized by the following stratigraphy, proceeding from the surface materials downward, unless noted otherwise below. Classification tables and charts used to determine the soil attributes are included in Appendix B.

Test Pit

The top of the concrete was uncovered in TP-1(Front Yard) at 73" bgs. The concrete protrudes 4" from the wall and extends 10" deep at this location.

Surface Cover

The surface cover for boring B-1 and B-2 consisted of grass cover and topsoil, approximately 3 to 4 inches thick.

Stratum 1: Sand (SP, SM)

Beneath the surface cover in each boring, a soil layer was encountered consisting of brown coarse-to-fine grained Sand with varying amounts of Silt and fine Gravel. This Sand layer extended from the bottom of the surface cover to approximately 13.5 feet below the ground surface (bgs) in boring B-1 and 15.5 bgs in boring in B-2.

The SPT-N values in this layer ranged from 4 to 26 blows per foot (bpf), which is indicative of very loose to medium Sand. The SPT N-values for Stratum 1 are summarized in the tables below.

Within the Sand layer in boring B-1, two Silt lenses were encountered at 7.58 and 9.67 feet bgs. These lenses ranged in thickness from 4 to 5 inches, and the Silt material can be classified as medium-dense in accordance with the blow counts recorded within these lenses.

Table 6.0-1: Very Loose to Loose SPT N-Values for Stratum 1

Soil Boring Location	USCS Group Symbol	Depth Below Ground Surface	SPT N-Values
B-1	SM, SP	0-4'	6-8
B-2	SM	0-4', 13.5-15.5'	4-10

Table 6.0-2: Medium-Dense SPT N-Values for Stratum 1

Soil Boring Location	USCS Group Symbol	Depth Below Ground Surface	SPT N-Values
B-1	SW, SP	4-13.5’	13-20
B-2	SM	4-13.5’	12-26

Stratum 2: Clay (CL, CH)

Beneath the granular material of Stratum 1, a cohesive soil layer was encountered consisting of grey or brown Clay with varying amounts of Silt and trace amounts of fine Sand and Gravel. This cohesive layer was encountered at approximately 13.5 feet bgs in boring B-1 and at 15.5 feet bgs in boring B-2. Both borings were terminated within this Clay layer at 27 feet bgs.

The SPT-N values in this layer typically ranged from 10 to 17 blows per foot (bpf), which is indicative of stiff to very stiff Clay. One outlying N-value of 6 bpf was recorded within this layer in boring B-1 at the 25-27-foot sampling interval (medium-soft Clay). The SPT N-values for Stratum 2 are summarized in the tables below.

Table 6.0-4: Medium-Soft SPT N-Values for Stratum 3

Soil Boring Location	USCS Group Symbol	Depth Below Ground Surface	SPT N-Values
B-1	CH	23.5-27’	6

Table 6.0-5: Stiff to Very Stiff SPT N-Values for Stratum 3

Soil Boring Location	USCS Group Symbol	Depth Below Ground Surface	SPT N-Values
B-2	CL	13.5-23.5’	14-16
B-2	CL	15.5-27’	10-17

Groundwater

Groundwater levels could not be measured during drilling in either boring, due to the use of water and drilling mud to advance the borings. Based on soil saturation levels, the groundwater table lies approximately between 4.2 and 4.3 feet bgs. Saturated soils were encountered in B-1 at 4.2 feet bgs at 08:35AM and in B-2 at 4.3 feet bgs at 9:57AM. It should be noted that the groundwater levels will vary with temperature, precipitation, and other climatic factors.

7.0 GEOTECHNICAL SUBSURFACE PARAMETERS

The geotechnical design parameters in this report are derived from the field program and are based on accepted geotechnical standards and practices. At the time of the geotechnical assessment, loading conditions and the final proposed grading plans were not available. Therefore, certain assumptions were made for the recommendations provided in this report.

Table 7.0-1 summarizes the recommended geotechnical design parameters for the various soil strata encountered at the Site. The values are based on review and interpretation of the subsurface field program and laboratory test data results.

Table 1806.2 of the 2018 International Building Code provides allowable coefficients of friction to be used in the evaluation of resistance to sliding.

Table 7.0-1: Geotechnical Design Parameters

Stratum	Unit Weight (pcf)	Friction Angle (Φ) (deg)	Cohesive Strength, c_u (psf)	Earth Pressure Coefficient Active (Ka)	Earth Pressure Coefficient Passive (Kp)	Net Allowable Foundation Pressure* (psf)	Lateral Bearing (psf/ft. bgs)
Native Medium-Dense to Dense Granular Soil (SP, SP-SM, SM) [SPT N > 10]	γ = 125 γ' = 63	32°	0	0.31	3.26	4,000	200
Native Loose Granular Soil (SP, SP-SM, SM) [SPT N ≤ 10]	γ = 105 γ' = 43	30°	0	0.33	3.00	2,500	150
Native Silt (ML) Medium [10 ≤ SPT N ≤ 30]	γ = 115 γ' = 53	28°	400	0.36	2.77	2,000*	100
Native Clay (CL) Stiff [8 < SPT N ≤ 30]	γ = 110 γ' = 48	-	1,500	-	-	2,000*	100
Native Clay (CL) Medium-Soft [4 ≤ SPT N ≤ 8]	γ = 100 γ' = 38	-	1,000	-	-	1,500*	75

Notations: γ = moist unit weight, γ ' = buoyant unit weight, and c_u = average undrained shear strength.

\+ Allowable foundation pressure is contingent upon either replacement of at least two feet of existing fill below the bottom of footing by a Controlled Fill, or upon confirmation that the field density of the existing fill material down to four feet below the bottom of footing meets 95% of the maximum dry density of the existing fill material observed in Modified Proctor Tests.

* These values are based on the 2018 International Building Code, New Jersey Edition, and adjusted for field conditions encountered. To increase the allowable foundation pressure above the values recommended in the table given above, further testing of soil will be required. In Cohesive soils, it should be noted that the shallow footing may fail under the settlement criteria before the footing pressure approaches the anticipated allowable bearing capacity. Allowable Foundation Pressure values assume the water table is below the influence depth of the foundation.

- Coefficient of earth pressure at rest may be computed using Jaky's equation, $K_o = 1 - \sin \phi'$.

8.0 STRUCTURAL INSPECTION

The following sections present the results of the structural inspection of the residential building at 30 Carlos Drive in Fairfield, New Jersey. The conclusions presented herein are derived from Matrix's geotechnical and structural investigation of the existing soils and building foundations and framing configurations, along with pertinent survey data as compiled by Matrix's team of land surveyors.

Matrix conducted a subsurface investigation that included both a test pit and soil borings to obtain maximum pertinent information regarding the existing site conditions (refer to Section 6.0 of this report). The test pit performed at the site exposed the exterior portion of the building's foundation wall footing, allowing for measurement of dimensions of the structure and assessment of the construction methods utilized. Two geotechnical borings were also conducted to gain further information regarding the existing soils beneath the site.

In addition to the geotechnical investigation, Matrix also conducted a structural site inspection to observe the existing foundation walls and framing of the building. Matrix's structural engineer was granted access to the residence's basement and garage to observe the building's foundation structure. Substructure composition was recorded, including beam/girder type, building dimensions, and spacing of structural components. Structural defects, if any, were also noted during the inspection and have been included within Section 8.3.

8.1 Existing Building Foundations

The two-story building at 30 Carlos Drive sits atop a finished basement with CMU foundation walls. The timber frame and joists of the building are supported by nominal 2x10 timber girders spanning the foundation walls.

The basement, measuring approximately 26'-7" long x 52'-10" wide, encompasses the entire habitable footprint of the residence and supports two floors above. The foundation walls spanning the perimeter of the basement consist of 8x8x18 CMU blocks ranging from 85" to 85.5" in height. The basement floor measured approximately 8'-2" below the first-floor surface above. Since the basement is a finished area, most of the CMU walls are covered in a stucco veneer on their interior face. Apparent basement renovations were ongoing at the time of the inspection, as most partition walls still had exposed timber studs and the CMU walls along the front (east) wall were not coated with paint or stucco.

The subfloor of the first floor above the basement consists of nominal 2x10 timber joists typically spaced 16” on center and running east to west (front to rear of the building). These joists are supported by the perimeter CMU foundation walls of the basement as well as timber girders running north to south and spanning the width of the basement in the middle of the area. The two girders, which are nearly in line with each other along the middle of the space, each consist of (3) nominal 2x10 timber members. The girders are supported at one end by the CMU foundation walls, and at the other end (in the center of the basement) by a column of stacked CMU blocks. Both girders are further supported along their lengths by 4” diameter steel posts which extend down through the basement floor. The longest clear span of the north girder measured approximately 11’-6”, while the south girder’s longest clear span was approximately 9’-8” long.

Below the foundation walls in the southeast corner of the basement, an approximately 16” wide concrete spread footing was revealed during the test pit excavation program. Based on our findings within the test pits and from conventional foundation construction, Matrix utilized a 16” wide footing as a minimum value for analysis, but believes the actual footings for the building to likely range from16” to 24” in width. Prior to raising the house, Matrix recommends that the contractor confirm the foundation size and bearing adequacy with multiple test pits around the building perimeter.

The garage area is located north of the basement, and the two areas are separated by a staircase that leads to the first floor of the house. The garage consists of a concrete slab on grade with apparent CMU walls measuring 32.5” high and spanning the perimeter (contractor to confirm wall type prior to raising the building). The wall along the north edge of the garage includes a small 1” notch located 4” below the top of the wall. Timber stud walls make up the garage frame above the foundation walls. The ceiling of the garage measured approximately 8’-9” above the garage floor. There is no floor above the garage, but an attic was observed below the roof in this area.

Along the front of the building, the second floor overhangs the first floor by about 25.5”, and extends to 50” in front of the house’s front entrance. On either side of the front entrance stairs, an encased column has been installed to support the overhang of the second floor. The encasement prevented identification of the exact type of columns used for the building, but the columns were seen to extend down into the ground on presumed spread footings.

8.2 Existing Equipment

Various pieces of equipment and machinery were observed within the building's basement at the time of the inspection. Two sump pumps were observed – one in the southwest corner and one along the east wall near the center of the basement area. Also observed in the basement, in the same room as the eastern sump pit, was a water heater situated on the floor, a washer and dryer also on the floor, a boiler raised 28" off the floor with CMU blocks, and a gas meter raised 54" off the floor. Multiple PVC and metal pipes were also observed running along, through, and above the unfinished walls of the basement.

The only equipment observed within the garage was an electrical panel raised approximately 36" off the ground on the north wall. Multiple electrical conduits run off the panel and along the garage walls to feed the rest of the building.

Outside the building, along the south wall, two air conditioning units were observed. The units were raised 8" to 9" above the exterior grade with CMU blocks on concrete pads. Also, a generator was seen adjacent to the north edge of the rear deck. The generator was on a timber platform which raised the bottom of the unit 48" off the exterior grade.

8.3 Site Observations

Within the completed finished portion of the building's basement, two large openings were observed in the ceiling, exposing the subfloor above. The joists visible in each of of these openings had holes cut into their depth to allow for metal conduit to run through.

A CMU block exhaust chimney was observed adjacent to the boiler in the basement and next to the CMU column for the first-floor support girders. A second, brick chimney was observed along the north wall of the basement, with a fireplace noted in the basement. This chimney can be seen extending out through the lower roof of the house from the exterior.

The basement also contained baseboard heaters running along some of the walls of the area, including the east and west perimeter walls and the unfinished timber stud walls in the south half of the basement.

In the southeast corner of the garage, multiple cracks and water staining were observed in the wall and the drywall ceiling. Some exposed brick was also seen in the wall, likely the exterior of the brick chimney. The southeast corner of the garage also exhibited some separation, as a small gap was seen between the two intersecting walls.

A timber deck was observed spanning the width of the house in the rear of the building. The deck is flush with the first floor of the house and approximately 71" above adjacent exterior grade on average. The consists of nominal 2x8 floor joists spaced 12" on center. The joists area supported at their east end by the rear CMU foundation wall of the house and at the west end by a timber girder [(2) nominal 2x10 members] on timber posts embedded in Sonotube footings.

8.4 Elevation Requirements

The FEMA 100-year flood elevation at 30 Carlos Drive is El. +171 (NAVD88). As per the New Jersey Department of Community Affairs (DCA), and in accordance with the New Jersey Flood Hazard Area Control Act, the lowest floor of newly elevated buildings must be at least 3 feet above the base flood elevation. Therefore, the new first floor elevation must be at El. +174 or higher to meet the requirements set forth in the program.

The current first-floor elevation at the Site is at El. +173.49, with the adjacent garage floor at El. +169.92 and the finished basement floor at El. +165.32. To achieve the elevation requirements, the existing building would need to be raised at least 0.6 feet to elevate the existing first floor 3 feet above the BFE.

8.5 Recommendations for Building Elevation

Matrix recommends that the existing foundation system of the residential building at 30 Carlos Drive be kept and extended to achieve the required design flood elevation. The existing CMU foundation walls are expected to provide sufficient support for the additional height of the newly raised building. Based on loading estimation and analysis for the existing building, Matrix estimates that the anticipated additional dead load of the required new courses of CMU would remain under an allowable bearing capacity of 2,500 psf for the shallow concrete strip footings at the Site.

In accordance with NFIP requirements, it is required that the existing basement area be filled in to match the lowest adjacent exterior grade following raising. The ground-level space beneath the newly raised building can be used for storage at the resident's discretion. Raising the house by the minimum 0.6 feet will result in a loss of habitable area for the residence, as the existing basement floor will be partially filled in and can no longer be used for living space (ceiling too low and floor below the design flood elevation).

The most feasible method of elevation for the building consists of jacking up the entire residential structure from below using steel beams and jack posts. The building will then sit atop temporary cribbing while the existing CMU and concrete cellar/crawl space walls are heightened with additional courses of masonry block units. Additional vertical reinforcement would need to be installed in ungrouted masonry cells to properly transfer loads through the new heightened wall to the existing wall, and horizontal ladder reinforcement should be installed at a minimum of every other course. The garage area does not need to be raised, and the floor can be kept at ground level, as there is no habitable space above. The existing brick chimney in the garage/basement will also require extending during raising of the house to keep the top of the chimney above the roof level.

The existing steel post columns intermittently supporting the existing building's girders must be removed and replaced by new concrete or masonry block columns. Also, the two exterior columns supporting the second-floor overhang in the front of the building must be removed and replaced with longer columns of similar type. These new columns will need to include a spread footing beneath to sufficiently support the building loads.

Raising of the building should be undertaken with special attention to preserve the existing brick façade covering the timber frame on the first floor of the house. If the façade is kept in place during raising, the process is liable to lead to some cracking in the existing façade. Alternatively, the brick cover can be removed prior to raising, then replaced with a similar or more flexible finish after completion of the elevation process. The materials and labor involved in exterior façade restoration of the house have not been included in the scope of this project.

Within the new foundation walls, permanent openings are required to allow floodwater to enter the ground level and equalize the hydrostatic pressure during a flood event. As per the 2018 International Residential Code, New Jersey Edition, the total net area of non-engineered openings must comprise at least 1 square inch for every square foot of enclosed space within the building's ground floor. This equates to approximately 12.64 square feet of total flood openings in the building's new foundation walls. Additionally, a minimum of two openings must be provided for each enclosed area of the new ground floor. These openings must be located no higher than one foot above the adjacent finished exterior grade along the building perimeter. Matrix recommends the use of engineered openings in lieu of non-engineered openings to maximize efficiency and minimize the quantity of openings required.

Additionally, any service equipment, whether outside or in the cellar, such as air conditioning, heat pump compressors, gas meters, electric meters, and hot water heaters, must be elevated 3 feet above the BFE. For interior elements, this may include relocation to an upper floor and thus less usable living space. For this residence, the hot water heater, boiler, and gas meter in the basement, and the electrical panel in the garage, would require elevating 3 feet above the BFE onto the raised first floor. The 2 exterior air conditioning units, and the generator, would also require elevating 3 feet above the BFE on new or extended exterior platforms.

9.0 CLOSURE

This report has been prepared to assist the State of New Jersey Department of Community Affairs with the structural and geotechnical evaluation of the residential building 30 Carlos Drive in Fairfield, New Jersey. The conclusions and recommendations provided within this report were prepared based on our understanding of the project and through the application of generally accepted engineering practices. No warranties, expressed or implied, are made. Matrix should be notified of any changes to the existing building foundation system or if subsurface conditions differing from those described herein are encountered, so the impact on the geotechnical and/or structural recommendations can be evaluated.

10.0 REPRESENTATIVE SITE PHOTOS

Structural Inspection Photos

Photo 1. 30 Carlos Drive (Front of Building – South Side, Two-Story)

Photo 2. 30 Carlos Drive (Front of Building – North Side, One Story & Garage)

Photo 3. Rear of Building with Timber Deck

Photo 4. Column Supporting Second Floor Overhang in Front of Building (Looking West)

Photo 5. Finished Basement (Looking Southwest)

Photo 6. Unfinished Partition Wall in Basement with Girder & Steel Post (Looking Southeast)

Photo 7. South Girder & Steel Post in Basement (Looking North)

Photo 8. CMU Foundation Walls in Basement with Gas Meter & Sump Pit (Looking East)

Photo 9. Hot Water Heater in Basement (Looking Northeast)

Photo 10. Boiler in Basement (Looking Northwest)

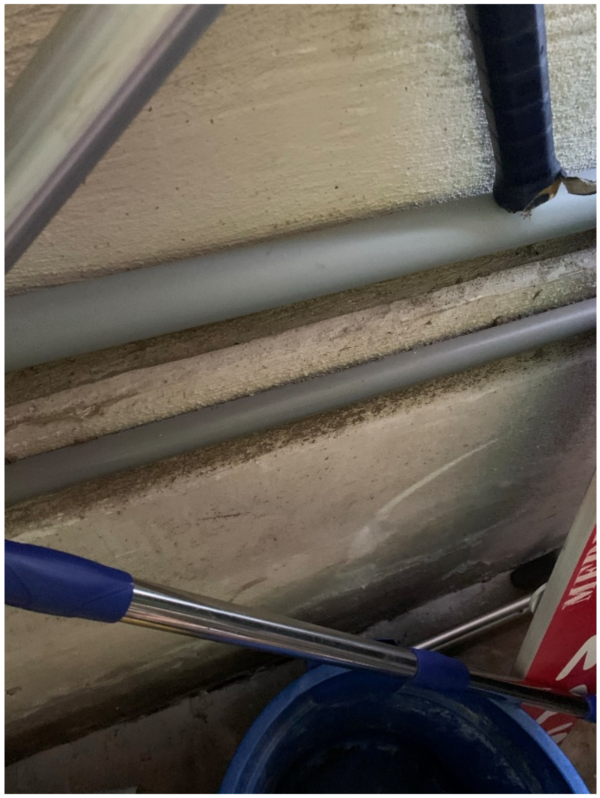


Photo 11. Typical Foundation Wall Along Garage Perimeter

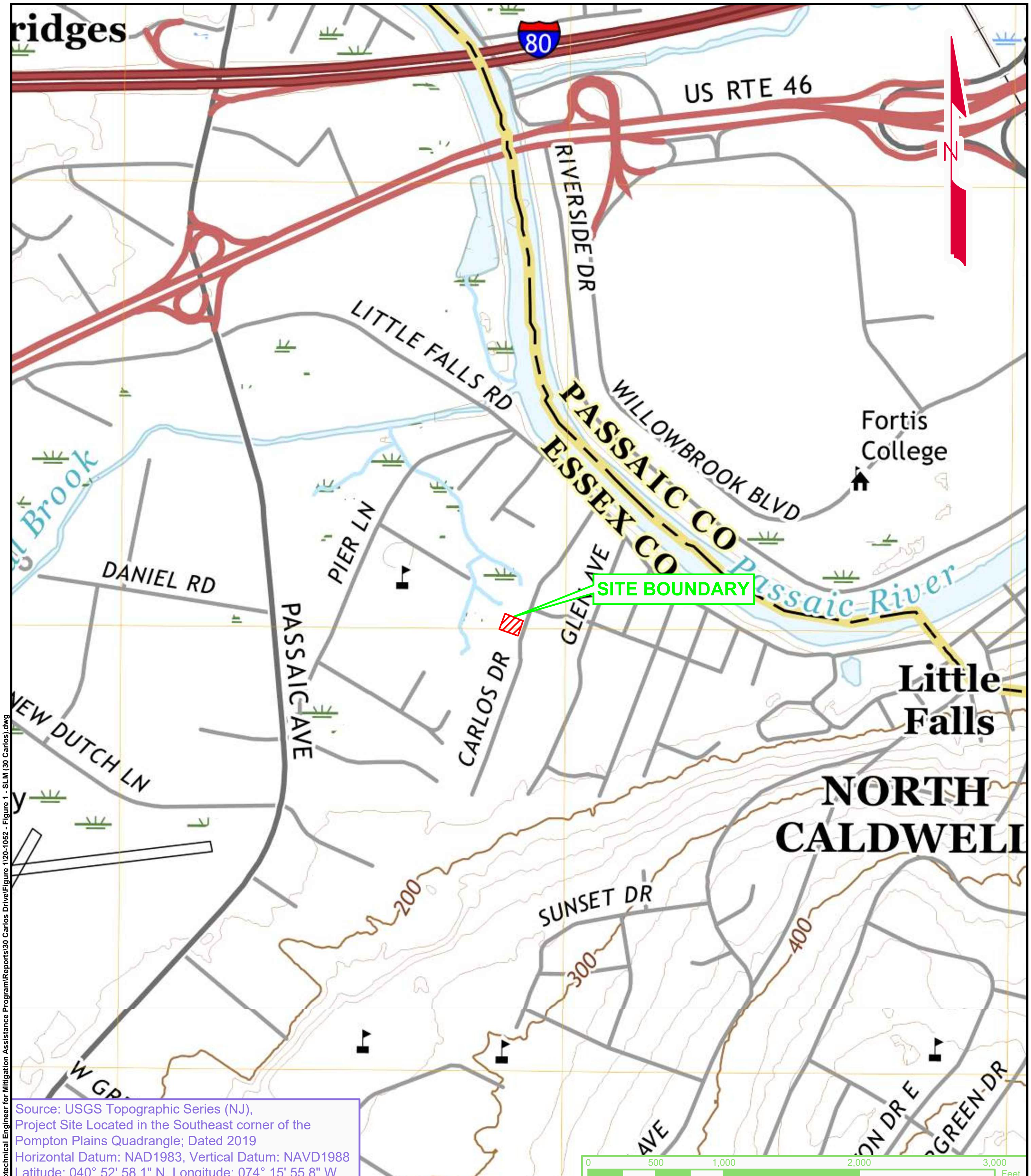
Photo 12. Cracks & Water Staining on South Wall of Garage (Southeast Corner)

Test Pit Photos

Photo 13. Test Pit TP-1 Location (Front of Building – Southeast Corner of Basement)

Photo 14. Test Pit TP-1 Foundation Conditions

FIGURES



Source: USGS Topographic Series (NJ),
Project Site Located in the Southeast corner of the Pompton Plains Quadrangle; Dated 2019
Horizontal Datum: NAD1983, Vertical Datum: NAVD1988
Latitude: 040° 52' 58.1" N, Longitude: 074° 15' 55.8" W

SITE LOCATION MAP

MATRIXNEWORLD
Engineering Progress

Matrix New World Engineering, Land Surveying and Landscape Architecture, P.C.
26 Columbia Turnpike
Florham Park, New Jersey 07932
WBE / DBE / SBE

Tel: 973-240-1800
Fax: 973-240-1818
www.matrixneworld.com

NJ DEPARTMENT OF COMMUNITY AFFAIRS
GEOTECHNICAL AND STRUCTURAL ASSESSMENT REPORT
30 CARLOS DRIVE
FAIRFIELD, NEW JERSEY 07004

SCALE:	PROJECT NO.:	DATE:	FIGURE NO.:
1" = 1,000'	20-1052	JUNE 2021	1

NOTES:

1. THIS FIGURE IS BASED ON IMAGERY PROVIDED BY GOOGLE EARTH AND TAX CARDS FROM FAIRFIELD TOWNSHIP, NEW JERSEY.
2. BORING LOCATIONS WERE IDENTIFIED IN THE FIELD BY MATRIX PERSONNEL BY TAPING AND LINE OF SIGHT MEASUREMENTS.
3. THE BORINGS WERE PERFORMED BY BORING BROTHERS, INC. ON MAY 12 THROUGH 13, 2021 UNDER THE DIRECTION OF A MATRIX REPRESENTATIVE.
4. ALL ELEVATIONS SHOWN ON THIS PLAN REFER TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88).

LEGEND

B-# AS-DRILLED BORING LOCATION

TP-# TEST PIT LOCATION

0 10 20 30 40 80

SCALE: 1" = 40'

NO.	DESCRIPTION	DATE:	BY:	APR:

REVISIONS

DESIGNED BY: WB | REVIEWED BY: MS | RELEASED BY: MS

MATRIXNEWWORLD
Engineering Progress

Matrix New World Engineering, Land Surveying and Landscape Architecture, P.C.
26 Columbia Turnpike
Florham Park, New Jersey 07932
WBE / DBE / SBE
Tel: 973-240-1800
Fax: 973-240-1818
www.matrixneworld.com

AS-DRILLED BORING LOCATION PLAN

NJDCA GEOTECHNICAL ENGINEER FOR MITIGATION ASSISTANCE PROGRAM

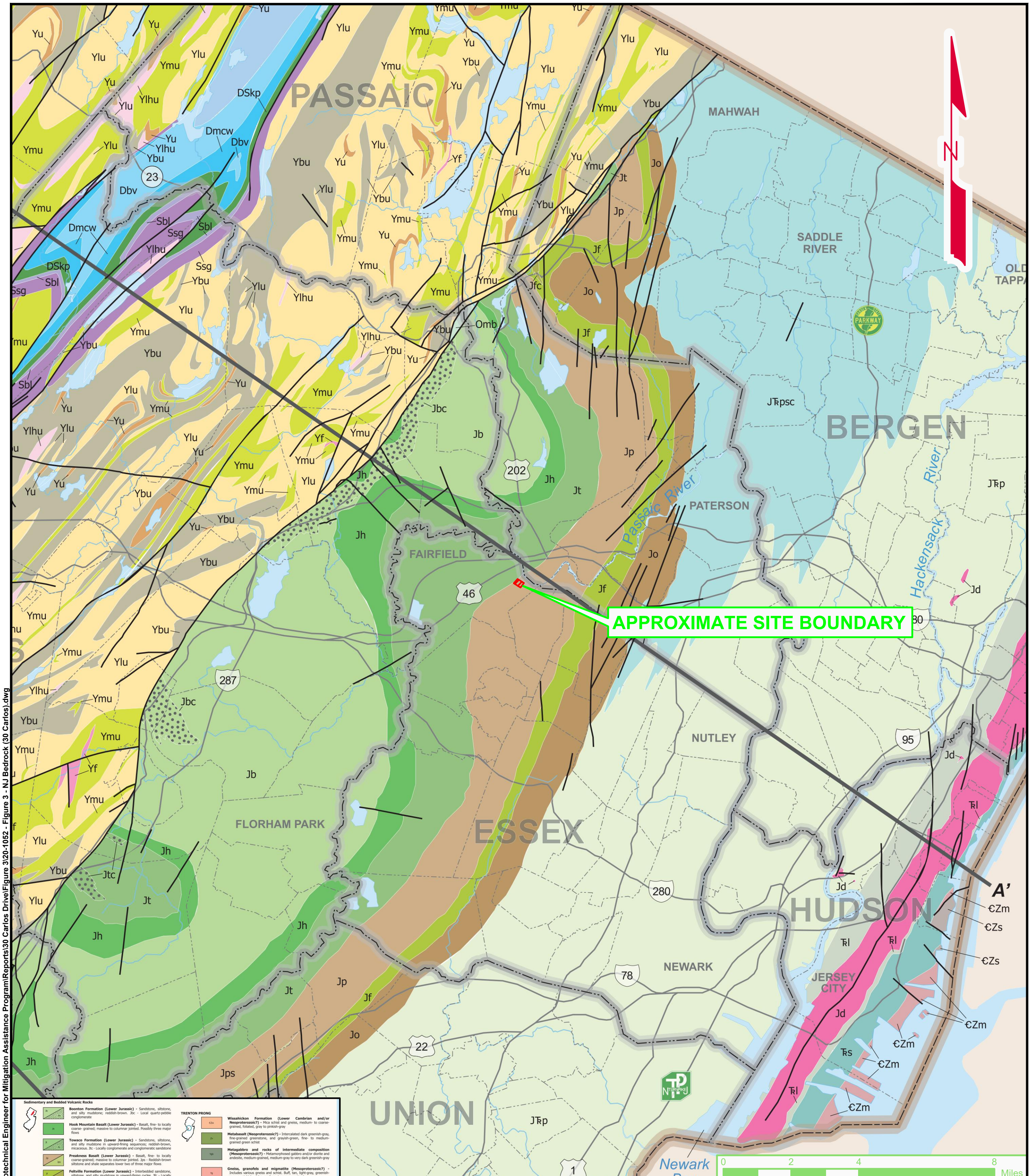
30 CARLOS DRIVE
FAIRFIELD, NJ 07004

PROJECT NUMBER: 20-1052

SCALE: AS NOTED

DATE: JUNE 2021

2



BEDROCK GEOLOGY LOCATION MAP

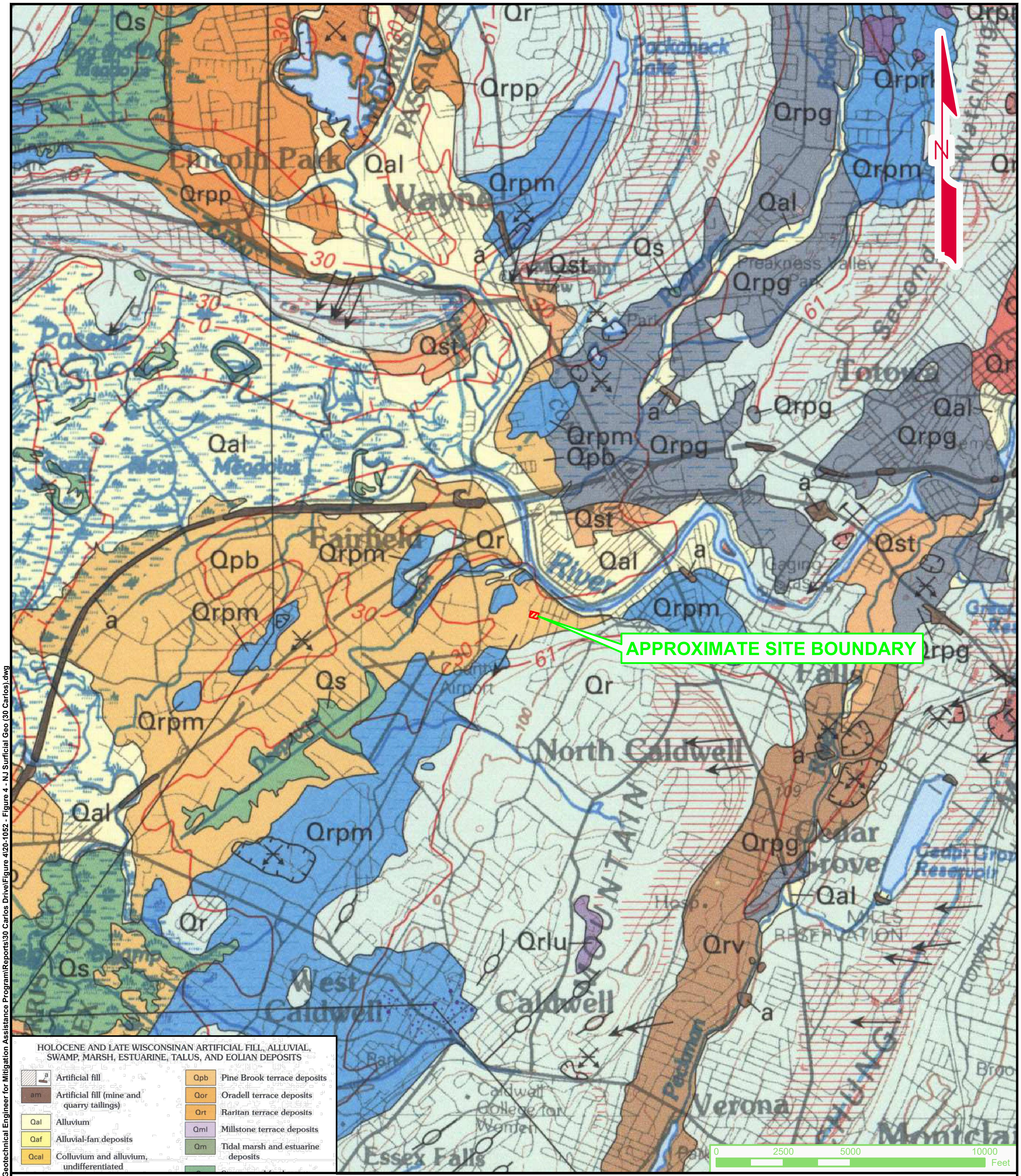
MATRIXNEWORLD
Engineering Progress

Matrix New World Engineering, Land Surveying and Landscape Architecture, P.C.
26 Columbia Turnpike
Florham Park, New Jersey 07932
WBE / DBE / SBE

Tel: 973-240-1800
Fax: 973-240-1818
www.matrixneworld.com

NJ DEPARTMENT OF COMMUNITY AFFAIRS
GEOTECHNICAL AND STRUCTURAL ASSESSMENT REPORT
30 CARLOS DRIVE
FAIRFIELD, NEW JERSEY 07004

SCALE:	PROJECT NO.:	DATE:	FIGURE NO.:
1" = 4 Miles	20-1052	JUNE 2021	3



SURFICIAL GEOLOGY LOCATION MAP

MATRIXNEWORLD
Engineering Progress

Matrix New World Engineering, Land Surveying and Landscape Architecture, P.C.
26 Columbia Turnpike
Florham Park, New Jersey 07932
WBE / DBE

Tel: 973-240-1800
Fax: 973-240-1818
www.matrixneworld.com

NJ DEPARTMENT OF COMMUNITY AFFAIRS
GEOTECHNICAL AND STRUCTURAL ASSESSMENT REPORT
30 CARLOS DRIVE
FAIRFIELD, NEW JERSEY 07004

SCALE:	PROJECT NO.:	DATE:	FIGURE NO.:
1" = 5000'	20-1052	JUNE 2021	4

APPENDIX A

SOIL BORING & TEST PIT LOGS

MATRIXNEWORLD
Engineering Progress

BORING LOG

BORING NO.: **B-1**

SHEET **1** OF **1**

PROJECT NO.: **20-1052** PROJECT: **NJDCA Geotechnical Engineer for Mitigation Assistance Program** DATE: **5/12/21**
PROJECT LOCATION: **Fairfield, NJ** BORING LOCATION: **30 Carlos Drive, Driveway**
DRILLING EQUIPMENT: **CME 55** ANGLE: **-90.0** DIR.: **------** ELEV.: DATUM: **NAVD88**
DRILLING CONTRACTOR: **Boring Brothers, Inc.** DRILLER: **R. Dollar** INSPECTOR: **D. Alia**

CASING and HAMMER				SAMPLER and HAMMER				GROUNDWATER LEVELS			
Type	I.D.	Weight	Drop	Type	I.D.	Weight	Drop	Date	Time	Depth	Casing Depth
Auto		**140 lbs**	**30"**	**AUTO**		**140 lbs**	**30"**	**5/12/21**	**8:35 am**	**4.2**	
FJ Steel	**4"**			**SS**	**1 3/8"**						

Depth Feet (Elev.)	CASING Blows/ Foot	SAMPLE No.	Type	Depth Feet	Blows/6" (REC. %) [RQD %]	Graphic Symbol	Description Of Material	Laboratory Tests
		S-1	SS	0-2	2-3-3-3 (8%)		3" Topsoil S-1: Brown-mf SAND, trace roots, dry (SP)	
		S-2	SS	2-4	2-3-5-7 (46%)		S-2: Brown-Black mf SAND and Silt, trace fine Gravel, moist (SM)	
5		S-3	SS	4-6	7-8-10-10 (83%)		S-3: Brown cmf SAND, trace fine Gravel, trace Silt, wet (SW)	
	4" Casing	S-4	SS	6-8	10-11-9-8 (83%)		S-4A (top 19"): Same as Above, wet (SW) S-4B (Bottom 5"): Brown SILT, trace fine Sand, dry (ML)	
		S-5	SS	8-10	8-8-8-8 (92%)		S-5A (Top 18"): Brown mf SAND, trace fine Gravel, trace Silt, dry (SP) S-5B (Bottom 4"): Gray SILT, trace fine Sand, dry (ML)	
10	4" Casing	S-6	SS	10-12	6-7-6-6 (100%)		S-6: Brown cmf SAND, trace Silt, wet (SW)	
15		S-7	SS	15-17	8-7-7-9 (79%)		S-7: Gray CLAY & Silt, trace fine Sand, moist (CL) WC: 18.9%, LL: 31, PL: 17, PI: 14	Atterberg Limits
20		S-8	SS	20-22	8-8-8-9 (83%)		S-8: Gray CLAY & Silt, trace fine Sand, trace fine Gravel, dry (CL)	
25		S-9	SS	25-27	2-3-3-2 (100%)		S-9: Gray Silty CLAY, dry (CH) WC: 34.6%, LL: 51, PL: 21, PI: 30	Atterberg Limits
							Bottom of Borehole @ 27 ft.	

MATRIXNEWORLD
Engineering Progress

BORING LOG

BORING NO.: **B-2**

SHEET **1** OF **1**

PROJECT NO.: **20-1052** PROJECT: **NJDCA Geotechnical Engineer for Mitigation Assistance Program** DATE: **5/12/21**
PROJECT LOCATION: **Fairfield, NJ** BORING LOCATION: **30 Carlos Drive, Driveway**
DRILLING EQUIPMENT: **CME 55** ANGLE: **-90.0** DIR.: **------** ELEV.: DATUM: **NAVD88**
DRILLING CONTRACTOR: **Boring Brothers, Inc.** DRILLER: **R. Dollar** INSPECTOR: **D. Alia**

CASING and HAMMER				SAMPLER and HAMMER				GROUNDWATER LEVELS			
Type	I.D.	Weight	Drop	Type	I.D.	Weight	Drop	Date	Time	Depth	Casing Depth
Auto		**140 lbs**	**30"**	**AUTO**		**140 lbs**	**30"**	**5/12/21**	**9:57 am**	**4.3**	
FJ Steel	**4"**			**SS**	**1 3/8"**						

Depth Feet (Elev.)	CASING Blows/ Foot	SAMPLE No.	Type	Depth Feet	Blows/6" (REC. %) [RQD %]	Graphic Symbol	Description Of Material	Laboratory Tests
		S-1	SS	0-2	2-2-3-4 (71%)		4" Topsoil S-1: Brown mf* SAND, little Silt, trace fine Gravel, dry (SM)	
		S-2	SS	2-4	2-2-2-2 (33%)		S-2: Dark Brown mf* Sand, some Silt, moist (SM)	
5		S-3	SS	4-6	8-7-5-6 (75%)		S-3: Brown mf* SAND, some Silt, wet (SM) WC: 15.3%, Gravel: 0.0%, Sand:76.9% , Fines: 23.1%	Sieve
		S-4	SS	6-8	8-8-13-12 (100%)		S-4: Same as Above, wet (SM)	
		S-5	SS	8-10	10-8-8-8 (100%)		S-5: Dark Brown mf SAND, some Silt, wet (SM)	
10		S-6	SS	10-12	14-14-12-11 (100%)		S-6: Brown mf SAND, little Silt, trace fine Gravel, moist (SM)	
15		S-7	SS	15-17	5-3-6-7 (75%)		S-7A (Top 6"): Dark Brown mf SAND, little Silt, wet (SM) S-7B (Bottom 12"): Gray Silty CLAY, trace fine Sand, wet (CL)	
20		S-8	SS	20-22	4-4-6-18 (83%)		S-8: Brown CLAY, trace fine Sand, trace fine Gravel, wet (CL) WC: 22.7%, Gravel: 4%, Sand: 8%, Fines: 88%, <2 µm: 39%	Sieve; Hydrometer
25		S-9	SS	25-27	7-8-9-9 (92%)		S-9: Gray Silty CLAY (CL)	
							Bottom of Borehole @ 27 ft.	

MATRIXNEWORLD
Engineering Progress

TEST PIT LOG

TEST PIT NO.: **TP-1**

SHEET **1** OF **1**

PROJECT NO.: **20-1052** PROJECT: **NJDCA Geotechnical Engineer for Mitigation Assistance Program** DATE: **5/13/2021**

PROJECT LOCATION: **Fairfield, NJ** ELEV.: TIME STARTED: **7:30:00 AM**

TEST PIT LOCATION: **30 Carlos Drive (Front Yard)** DATUM: **NAVD88** TIME FINISHED: **9:00:00 AM**

CONTRACTOR: **Boring Brothers, Inc.** GROUNDWATER LEVEL:

EQUIPMENT: **Bobcat E55** OPERATOR: **Steve** INSPECTOR: **A. Bangar**

Depth Inches (Elev)	No.	Depth Inches	Graphic Symbol	Description Of Material	Laboratory Tests
		0-73		Brown mf SAND and Silt, some fine Gravel, dry (SP)	
5		5		Abandoned terracotta pipe at ~5" bgs	
10					
15					
20					
25					
30					
35					
40					
45					
50					
55					
60					
65		63-73		Top of concrete encountered at 63" bgs, protrudes 4" from the face of the wall and extends 10" downward.	
70					
				Bottom of Test pit @ 73 in. Test Pit Backfilled.	

LOG NOTATION

Sample Classifications

SS = Split Spoon

NR = No Recovery

NX = Rock Core

SH = Shelby Tube

REC = Soil Recovery

RQD = Rock Quality Designation

Sand Classifications

c = Coarse

m = Medium

f = Fine

* = Predominant Grain Size

Soil Properties

WC = Water Content

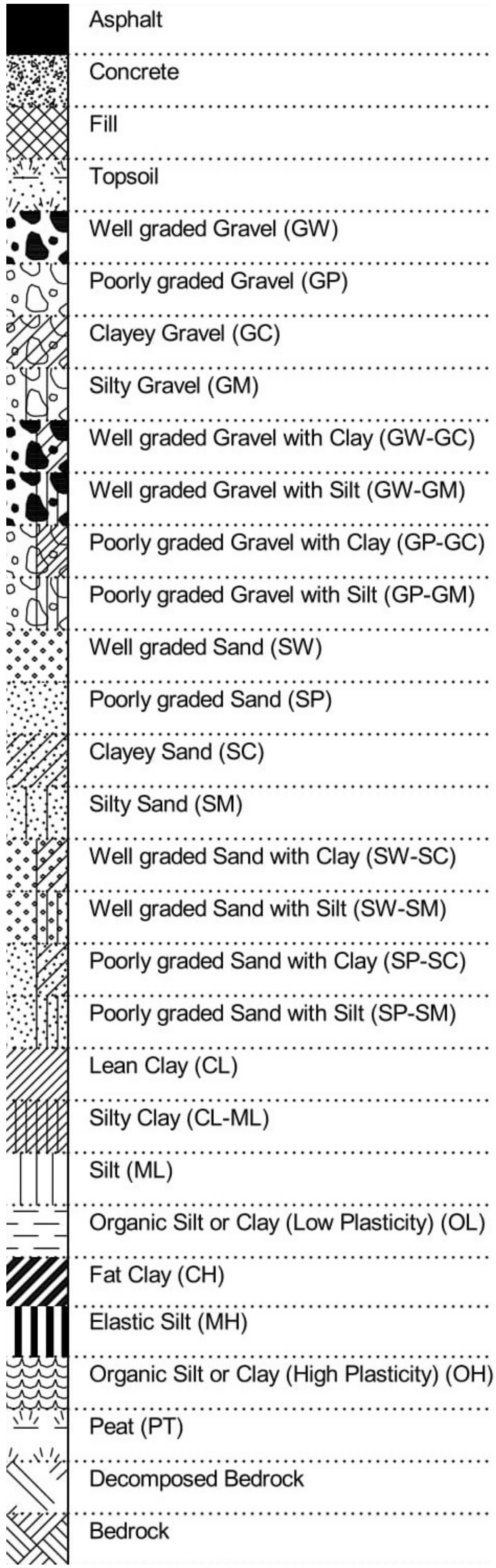
PL = Plastic Limit

LL = Liquid Limit

PI = Plasticity Index

OC = Organic Content

LOG GRAPHICAL LEGEND



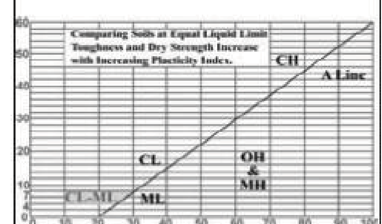
APPENDIX B

SOIL CLASSIFICATION TABLES

MAJOR DIVISIONS (1)	(2)	(2)	GROUP SYMBOLS (3)	TYPICAL NAMES (4)	FIELD IDENTIFICATION PROCEDURES (EXCLUDING PARTICLES LARGER THAN 3 IN. AND BASING FRACTIONS ON ESTIMATED WEIGHTS) (5)	INFORMATION REQUIRED FOR DESCRIBING SOILS (6)	LABORATORY CLASSIFICATION CRITERIA (7)
Coarse-grained Soils More than half of material is larger than No. 200 sieve size. The No. 200 sieve size is about the smallest visible to the naked eye.	Gravels More than half of coarse fraction is larger than No. 4 sieve size. (For visual classification. the 1/4-in. size may be used as equivalent to the No. 4 sieve size.)	Clean Gravels (Little or no fines)	GW	Well-graded gravels, gravel-sand mixture, little or no fines.	Wide range in grain size and substantial amounts of all intermediate particle sizes.	For undisturbed soils add information on stratification, degree of compactness, cementation, moisture condition, and drainage characteristics.	Use grain-size curve in identifying the fractions as given under field identification. Determine percentage of gravel and sand from grain-size curve. Depending on percentage of fine (fraction smaller than No. 200 sieve size) coarse-grained soils are classified as follows: Less than 5% GW, GP, SW, SP, More than 12% GM, GC, SM, SC. 5% to 12% Borderline cases requiring use of dual symbols. $C_u = \frac{D_{60}}{D_{10}}$ Greater than 4 $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ Between 1 and 3
			GP	Poorly graded gravels or gravel-sand mixture, little or no fines.	Predominantly one size or a range of sizes with some intermediate sizes missing.		Not meeting all gradation requirements for GW
		Gravels with Fines (Appreciable amount of fines)	GM	Silty gravels, gravel and silt mixtures.	Nonplastic fines or fines with low plasticity (for identification procedures see ML below).	Give typical name; indicate approximate percentages of sand and gravel, maximum size; angularity, surface condition, and hardness of the coarse grains; local or geologic name and other pertinent descriptive information; and symbol in parentheses.	Atterberg limits below "A" line or P1 less than 4 Above "A" line with P1 between 4 and 7 are borderline cases requiring use of dual symbols.
			GC	Clayey gravels, gravel and clay mixtures.	Plastic fines (for identification procedures see CL below).		Atterberg limits above "A" line with P1 greater than 7
	Sands More than half of coarse fraction is smaller than No .4 sieve size.	Clean Sand (Little or no fines)	SW	Well-graded sands, gravelly sands, little or no fines.	Wide range in grain size and substantial amounts of all intermediate particle sizes.		$Cu = \frac{D_{60}}{D_{10}}$ Greater than 6 $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ Between 1 and 3
			SP	Poorly graded sands or gravelly sands, little or no fines.	Predominantly one size or a range of sizes with some intermediate sizes missing.	Example: Silty sand, gravelly; about 20% hard, angular gravel particles 1/2-in. maximum size; rounded and subangular sand grains, coarse to fine; about 15% nonplastic fines with low dry strength; well compacted and moist in place; alluvial sand; (SM).	Not meeting all gradation requirements for SW
		Sands with Fines (Appreciable amount of fines)	SM	Silty sands, sand-silt mixtures.	Nonplastic fines or fines with low plasticity (for identification procedures see ML below).		Atterberg limits above "A" line or P1 less than 4 Limits plotting in hatched zone with P1 between 4 and 7 are borderline cases requiring use of dual symbols.
			SC	Clayey sands, sand-clay mixtures.	Plastic fines (for identification procedures see CL below).		Atterberg limits above "A" line with Pl greater than 7

MAJOR DIVISIONS (1)	(2)	GROUP SYMBOLS (3)	TYPICAL NAMES (4)	Identification Procedure on Fraction Smaller than No. 40 Sieve Size. Dry Strength (Crushing Characteristics)	Dilatancy (Reaction to shaking)	Toughness (Consistency near PL)	INFORMATION REQUIRED FOR DESCRIBING SOILS (6)	LABORATORY CLASSIFICATION CRITERIA (7)
Fine-grained Soils More than half of material is smaller than No. 200 sieve size.	Silts and Clays Liquid limit is less than 50	ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.	None to slight	Quick to slow	None	For undisturbed soils add information on structure, stratification, consistency in undisturbed and remolded states, moisture and drainage conditions	LIQUID LIMIT PLASTICITY CHART For laboratory classification of fine-grained soils
		CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays.	Medium to high	None to very slow	Medium		
	Silts and Clays Liquid limit is greater than 50	OL	Organic silts and organic silty clays of low plasticity.	Slight to medium	Slow	Slight	Give typical name; indicate degree and character of plasticity; amount and maximum size of coarse grains; color in wet condition; odor, if any; local or geologic name and other pertinent descriptive information; and symbol in parentheses.	
		MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts.	Slight to medium	Slow to none	Slight to medium		
		CH	Inorganic clays of high plasticity, fat clays.	High to very high	None	High		
		OH	Organic clays of medium to high plasticity, organic silts.	Medium to high	None to very slow	Slight to medium	Example: Clayey silt, brown; slightly plastic; small percentage of fine sand; numerous vertical root holes; firm and dry in place; loess; (ML)	
Highly Organic Soils		Pt	Peat and other highly organic soils.	Readily identified by color, odor, spongy feel and frequently by fibrous texture				

Plasticity Index



1. Boundary classifications: Soils possessing characteristics of two groups are designed by combinations of group symbols. For example GM-GC, well-graded gravel-sand mixture with clay binder.
2. All sieve sizes on this chart are U.S. standard.
3. Adopted by Corps of Engineers and Bureau of Reclamation, January 1952.

032058C

BURMISTER SOIL IDENTIFICATION METHOD

BURMISTER SOIL IDENTIFICATION METHOD

1. SOIL MATERIAL Composition, Gradation, and Plasticity Characteristics

a) Soil Components and Soil Fractions

Sieve	3"	1"	3/8"	No. 10 2 mm	No. 30	No. 60	No. 200 0.076 mm	0.02 mm
Granular Component Fractions	GRAVEL: coarse	medium	fine	SAND: coarse	medium	fine	SILT: coarse	fine
Clay Soil Components							CLAY-SOIL Defined and Named on a Plasticity Basis	

b) Identifying Terms for Granular Soils
Composition and Proportion Terms for Components

Component		Proportion Terms	Defining Range of Percentages
Principal Components- GRAVEL, SAND, SILT (all Uppercase)			50% or more
Minor Components-	Gravel	and	35 to 50%
	Sand	some	20 to 35%
	Silt	little	10 to 20%
		trace	1 to 10%

Gradation Terms for Granular Soils		ORGANIC SOILS
coarse to fine	all fractions more than 10%	Plasticity Basis, as
coarse to medium	fine less than 10%	
medium to fine	coarse less than 10%	Organic SILT, H. PI
medium	coarse and fine less than 10%	
fine	coarse and medium less than 10%	Organic SILT, L. PI
PLUS or MINUS signs used to indicate upper or lower limits.		

c) Identifying Terms for CLAY SOILS. Plasticity Basis for Combined Silt and Clay Components, Expressing the Relative Dominance of Clay

Overall Plasticity	Plasticity Index	Principal Component	Minor Component
Non-Plastic	0	SILT	Silt
Slight	1 to 5	Clayey SILT	Clayey Silt
Low	5 to 10	SILT & CLAY	Silt & Clay
Medium	10 to 20	CLAY & SILT	Clay & Silt
High	20 to 40	Silty CLAY	
Very High	more than 40	CLAY	

Example: Soil 60% coarse to fine Sand, 25% medium to fine Gravel, 15% Clayey Silt and color-brown.
Identification: Br. coarse to fine SAND, some medium to fine Gravel, little Clayey Silt.

References:
1) D. M. Burmister, "Principles and Techniques of Soil Identification" 29th Highway Research Board Proceedings, 1949.
2) "Identification and Classification of Soils – An appraisal and Statement of Principles", ASTM Special Technical Publication No. 113, 1951.

Field Classification of Soil Using the USCS

Apparent Density of Coarse-Grained Soils

SPT N-Value (corrected)	Apparent Density
0 - 4	Very loose
5 - 10	Loose
11 - 30	Medium Dense
31 - 50	Dense
> 50	Very Dense

Consistency of Fine-Grained Soils

SPT N-Value (uncorrected)	Consistency	Compressive Strength (ksf)	Results of Manual Manipulation
< 2	Very Soft	< 0.5	Specimen (height = twice the diameter) sags under its own weight; extrudes between fingers when squeezed
3 - 4	Soft	> 0.5 - 1.0	Speciment can be pinched in to between the thumb and forefinger; remolded by light finger pressure
5 - 8	Medium stiff	> 1.0 - 2.0	Can be imprinted easily with fingers; remolded by strong finger pressure
9 - 15	Stiff	> 2.0 - 4.0	Can be imprinted with considerable pressure from fingers or indented by thumbnail
16 - 30	Very stiff	> 4.0 - 8.0	Can be barely imprinted by pressure from the fingers or indented by thumbnail
> 30	Hard	> 8.0	Cannot be imprinted by fingers or difficult to indent by thumbnail

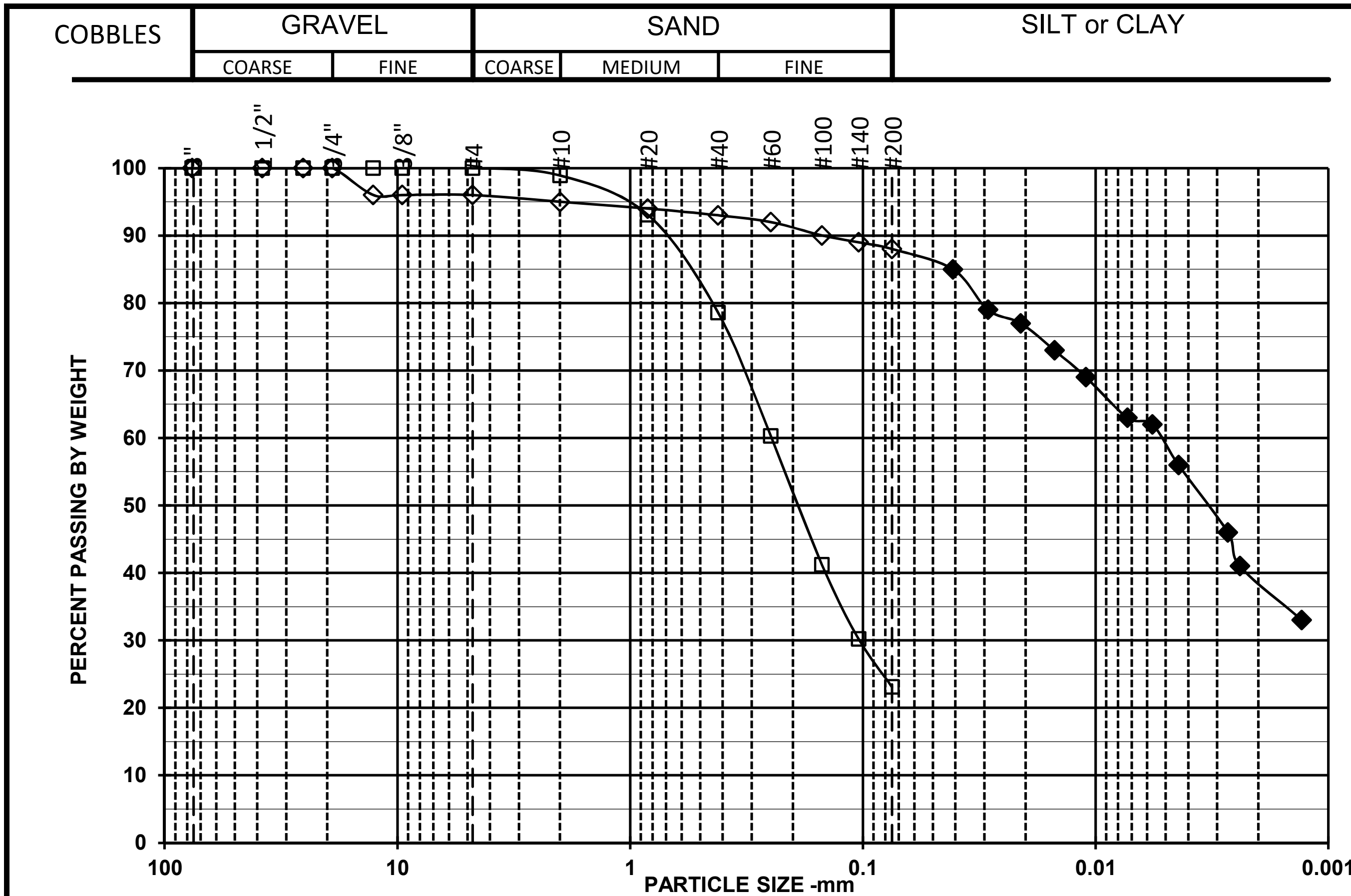
APPENDIX C

GEOTECHNICAL LABORATORY TESTING RESULTS

Matrix New World Engineering, P.C. #20-1052-003
NJDCA MAP - 30 Carlos Drive
LABORATORY TESTING DATA SUMMARY

BORING NO.	SAMPLE NO.	DEPTH (ft)	IDENTIFICATION TESTS							REMARKS
			WATER CONTENT (%)	LIQUID LIMIT (-)	PLASTIC LIMIT (-)	PLAS. INDEX (-)	USCS SYMB. (1)	SIEVE MINUS NO. 200 (%)	HYDROMETER % MINUS 2 μm (%)	
B-1	S-7	15-17	18.9	31	17	14	CL			
B-1	S-9	25-27	34.6	51	21	30	CH			
B-2	S-3	4-6	15.3				SM	23.1		
B-2	S-8	20-22	22.7				CL	88	39	

Note: (1) USCS symbol based on visual observation and Sieve and Atterberg limits reported.



Open Symbols: Sieve analysis by ASTM D6913
Filled symbols: Hydrometer analysis by ASTM D7928 corrected for complete sample

SYMBOL	w (%)	LL	PL	PI	USCS	AASHTO	USCS DESCRIPTION AND REMARKS	DATE
□	15.3				SM		Brown, Silty sand	06/29/21
◇	22.7				CL		Brown, Lean clay, Insufficient sample size	06/23/21
○								

Symbol	□	◇	○
Boring	B-2	B-2	
Sample	S-3	S-8	
Depth	4-6	20-22	
% +3"	0	0	
% Gravel	0	4	
% SAND	76.9	8	
%C SAND	1.1	1	
%M SAND	20.3	2	
%F SAND	55.5	5	
% FINES	23.1	88	
D_{100} (mm)	4.75	19.1	
D_{60} (mm)	0.247	0.005	
D_{30} (mm)	0.1		
D_{10} (mm)			
Cc			
Cu			

Sieve Size/ID #	Percent Finer Data		
6"	100.0	100	
4"	100.0	100	
3"	100.0	100	
1 1/2"	100.0	100	
1"	100.0	100	
3/4"	100.0	100	
1/2"	100.0	96	
3/8"	100.0	96	
#4	100.0	96	
#10	98.9	95	
#20	93.1	94	
#40	78.6	93	
#60	60.3	92	
#100	41.2	90	
#140	30.2	89	
#200	23.1	88	
5μ m		59	
2μ m		39	
1μ m		30	

Matrix New World Engineering, P.C. #20-1052-003

TerraSense, LLC #7783-21017

NJDCA MAP
30 Carlos Drive

PARTICLE SIZE DISTRIBUTION
ASTM D6913 & ASTM D7928

APPENDIX D

FEMA NFIP ELEVATION CERTIFICATE

OMB No. 1660-0008
Expiration Date: November 30, 2022

ELEVATION CERTIFICATE

Important: Follow the instructions on pages 1–9.

Copy all pages of this Elevation Certificate and all attachments for (1) community official, (2) insurance agent/company, and (3) building owner.

SECTION A – PROPERTY INFORMATION	FOR INSURANCE COMPANY USE
A1. Building Owner's Name [redacted]	Policy Number:
A2. Building Street Address (including Apt., Unit, Suite, and/or Bldg. No.) or P.O. Route and Box No. 30 Carlos Drive	Company NAIC Number:

City	State	ZIP Code
Town of Fairfield	New Jersey	07004-2117

A3. Property Description (Lot and Block Numbers, Tax Parcel Number, Legal Description, etc.)
Block 2501, Lot 32

A4. Building Use (e.g., Residential, Non-Residential, Addition, Accessory, etc.) Residential

A5. Latitude/Longitude: Lat. N40°52'58" Long. W74°15'55" Horizontal Datum: ☐ NAD 1927 ☒ NAD 1983

A6. Attach at least 2 photographs of the building if the Certificate is being used to obtain flood insurance.

A7. Building Diagram Number 2A

A8. For a building with a crawlspace or enclosure(s):

a) Square footage of crawlspace or enclosure(s) 1404.00 sq ft

b) Number of permanent flood openings in the crawlspace or enclosure(s) within 1.0 foot above adjacent grade 0

c) Total net area of flood openings in A8.b 0.00 sq in

d) Engineered flood openings? ☐ Yes ☒ No

A9. For a building with an attached garage:

a) Square footage of attached garage 290.00 sq ft

b) Number of permanent flood openings in the attached garage within 1.0 foot above adjacent grade 0

c) Total net area of flood openings in A9.b 0.00 sq in

d) Engineered flood openings? ☐ Yes ☒ No

SECTION B – FLOOD INSURANCE RATE MAP (FIRM) INFORMATION

B1. NFIP Community Name & Community Number	B2. County Name	B3. State
Fairfield, Township of	Essex	New Jersey

B4. Map/Panel Number	B5. Suffix	B6. FIRM Index Date	B7. FIRM Panel Effective/ Revised Date	B8. Flood Zone(s)	B9. Base Flood Elevation(s) (Zone AO, use Base Flood Depth)
34013C0019	G	04-03-2020	04-03-2020	AE	171 (NAVD88)

B10. Indicate the source of the Base Flood Elevation (BFE) data or base flood depth entered in Item B9:
☐ FIS Profile ☒ FIRM ☐ Community Determined ☐ Other/Source:

B11. Indicate elevation datum used for BFE in Item B9: ☐ NGVD 1929 ☒ NAVD 1988 ☐ Other/Source:

B12. Is the building located in a Coastal Barrier Resources System (CBRS) area or Otherwise Protected Area (OPA)? ☐ Yes ☒ No

Designation Date: ☐ CBRS ☐ OPA

ELEVATION CERTIFICATE

OMB No. 1660-0008
Expiration Date: November 30, 2022

IMPORTANT: In these spaces, copy the corresponding information from Section A.			FOR INSURANCE COMPANY USE
Building Street Address (including Apt., Unit, Suite, and/or Bldg. No.) or P.O. Route and Box No. 30 Carlos Drive			Policy Number:
City Town of Fairfield	State New Jersey	ZIP Code 07004-2117	Company NAIC Number

SECTION C – BUILDING ELEVATION INFORMATION (SURVEY REQUIRED)

C1. Building elevations are based on: ☐ Construction Drawings* ☐ Building Under Construction* ☒ Finished Construction

*A new Elevation Certificate will be required when construction of the building is complete.

C2. Elevations – Zones A1–A30, AE, AH, A (with BFE), VE, V1–V30, V (with BFE), AR, AR/A, AR/AE, AR/A1–A30, AR/AH, AR/AO. Complete Items C2.a–h below according to the building diagram specified in Item A7. In Puerto Rico only, enter meters.

Benchmark Utilized: CORS Network NGS Monuments Vertical Datum: NAVD 1988

Indicate elevation datum used for the elevations in items a) through h) below.

☐ NGVD 1929 ☒ NAVD 1988 ☐ Other/Source:

Datum used for building elevations must be the same as that used for the BFE.

Check the measurement used.

a) Top of bottom floor (including basement, crawlspace, or enclosure floor)	164.3	☒ feet	☐ meters
b) Top of the next higher floor	173.5	☒ feet	☐ meters
c) Bottom of the lowest horizontal structural member (V Zones only)	N/A	☐ feet	☐ meters
d) Attached garage (top of slab)	169.9	☒ feet	☐ meters
e) Lowest elevation of machinery or equipment servicing the building (Describe type of equipment and location in Comments)	164.3	☒ feet	☐ meters
f) Lowest adjacent (finished) grade next to building (LAG)	166.9	☒ feet	☐ meters
g) Highest adjacent (finished) grade next to building (HAG)	170.4	☒ feet	☐ meters
h) Lowest adjacent grade at lowest elevation of deck or stairs, including structural support	166.9	☒ feet	☐ meters

SECTION D – SURVEYOR, ENGINEER, OR ARCHITECT CERTIFICATION

This certification is to be signed and sealed by a land surveyor, engineer, or architect authorized by law to certify elevation information. *I certify that the information on this Certificate represents my best efforts to interpret the data available. I understand that any false statement may be punishable by fine or imprisonment under 18 U.S. Code, Section 1001.*

Were latitude and longitude in Section A provided by a licensed land surveyor? ☒ Yes ☐ No ☐ Check here if attachments.

Certifier's Name Frank J. Barlowski	License Number 24GS03973500		Place Seal Here
Title Professional Land Surveyor			
Company Name Matrix New World Engineering, Land Surveying and Architecture, P.C.			
Address 442 State Route 35, Second Floor			
City Eatontown	State New Jersey	ZIP Code 07724	
Signature	Date	Telephone	Ext.

Copy all pages of this Elevation Certificate and all attachments for (1) community official, (2) insurance agent/company, and (3) building owner.

Comments (including type of equipment and location, per C2(e), if applicable)

C2(e): Hot water heater on basement floor at Elev=164.3' (NAVD88)

ELEVATION CERTIFICATE

IMPORTANT: In these spaces, copy the corresponding information from Section A.			FOR INSURANCE COMPANY USE
Building Street Address (including Apt., Unit, Suite, and/or Bldg. No.) or P.O. Route and Box No. 30 Carlos Drive			Policy Number:
City Town of Fairfield	State New Jersey	ZIP Code 07004-2117	Company NAIC Number

SECTION E – BUILDING ELEVATION INFORMATION (SURVEY NOT REQUIRED) FOR ZONE AO AND ZONE A (WITHOUT BFE)

For Zones AO and A (without BFE), complete Items E1–E5. If the Certificate is intended to support a LOMA or LOMR-F request, complete Sections A, B,and C. For Items E1–E4, use natural grade, if available. Check the measurement used. In Puerto Rico only, enter meters.

E1. Provide elevation information for the following and check the appropriate boxes to show whether the elevation is above or below the highest adjacent grade (HAG) and the lowest adjacent grade (LAG).

a) Top of bottom floor (including basement, crawlspace, or enclosure) is ____________ ☐ feet ☐ meters ☐ above or ☐ below the HAG.

b) Top of bottom floor (including basement, crawlspace, or enclosure) is ____________ ☐ feet ☐ meters ☐ above or ☐ below the LAG.

E2. For Building Diagrams 6–9 with permanent flood openings provided in Section A Items 8 and/or 9 (see pages 1–2 of Instructions), the next higher floor (elevation C2.b in the diagrams) of the building is ____________ ☐ feet ☐ meters ☐ above or ☐ below the HAG.

E3. Attached garage (top of slab) is ____________ ☐ feet ☐ meters ☐ above or ☐ below the HAG.

E4. Top of platform of machinery and/or equipment servicing the building is ____________ ☐ feet ☐ meters ☐ above or ☐ below the HAG.

E5. Zone AO only: If no flood depth number is available, is the top of the bottom floor elevated in accordance with the community's floodplain management ordinance? ☐ Yes ☐ No ☐ Unknown. The local official must certify this information in Section G.

SECTION F – PROPERTY OWNER (OR OWNER'S REPRESENTATIVE) CERTIFICATION

The property owner or owner's authorized representative who completes Sections A, B, and E for Zone A (without a FEMA-issued or community-issued BFE) or Zone AO must sign here. The statements in Sections A, B, and E are correct to the best of my knowledge.

Property Owner or Owner's Authorized Representative's Name

Address	City	State	ZIP Code
Signature	Date	Telephone	

Comments

☐ Check here if attachments.

ELEVATION CERTIFICATE

IMPORTANT: In these spaces, copy the corresponding information from Section A.			FOR INSURANCE COMPANY USE
Building Street Address (including Apt., Unit, Suite, and/or Bldg. No.) or P.O. Route and Box No. 30 Carlos Drive			Policy Number:
City Town of Fairfield	State New Jersey	ZIP Code 07004-2117	Company NAIC Number

SECTION G – COMMUNITY INFORMATION (OPTIONAL)

The local official who is authorized by law or ordinance to administer the community's floodplain management ordinance can complete Sections A, B, C (or E), and G of this Elevation Certificate. Complete the applicable item(s) and sign below. Check the measurement used in Items G8–G10. In Puerto Rico only, enter meters.

G1. ☐ The information in Section C was taken from other documentation that has been signed and sealed by a licensed surveyor, engineer, or architect who is authorized by law to certify elevation information. (Indicate the source and date of the elevation data in the Comments area below.)

G2. ☐ A community official completed Section E for a building located in Zone A (without a FEMA-issued or community-issued BFE) or Zone AO.

G3. ☐ The following information (Items G4–G10) is provided for community floodplain management purposes.

G4. Permit Number	G5. Date Permit Issued	G6. Date Certificate of Compliance/Occupancy Issued

G7. This permit has been issued for: ☐ New Construction ☐ Substantial Improvement

G8. Elevation of as-built lowest floor (including basement) of the building: ____________ ☐ feet ☐ meters Datum ________

G9. BFE or (in Zone AO) depth of flooding at the building site: ____________ ☐ feet ☐ meters Datum ________

G10. Community's design flood elevation: ____________ ☐ feet ☐ meters Datum ________

Local Official's Name	Title
Community Name	Telephone
Signature	Date

Comments (including type of equipment and location, per C2(e), if applicable)

☐ Check here if attachments.

ELEVATION CERTIFICATE

BUILDING PHOTOGRAPHS

See Instructions for Item A6.

OMB No. 1660-0008
Expiration Date: November 30, 2022

IMPORTANT: In these spaces, copy the corresponding information from Section A.			FOR INSURANCE COMPANY USE
Building Street Address (including Apt., Unit, Suite, and/or Bldg. No.) or P.O. Route and Box No. 30 Carlos Drive			Policy Number:
City Town of Fairfield	State New Jersey	ZIP Code 07004-2117	Company NAIC Number

If using the Elevation Certificate to obtain NFIP flood insurance, affix at least 2 building photographs below according to the instructions for Item A6. Identify all photographs with date taken; "Front View" and "Rear View"; and, if required, "Right Side View" and "Left Side View." When applicable, photographs must show the foundation with representative examples of the flood openings or vents, as indicated in Section A8. If submitting more photographs than will fit on this page, use the Continuation Page.

Photo One

Photo One Caption Front View

Clear Photo One

Photo Two

Photo Two Caption Rear View

Clear Photo Two

ELEVATION CERTIFICATE

BUILDING PHOTOGRAPHS
Continuation Page

OMB No. 1660-0008
Expiration Date: November 30, 2022

IMPORTANT: In these spaces, copy the corresponding information from Section A.			FOR INSURANCE COMPANY USE
Building Street Address (including Apt., Unit, Suite, and/or Bldg. No.) or P.O. Route and Box No. 30 Carlos Drive			Policy Number:
City Town of Fairfield	State New Jersey	ZIP Code 07004-2117	Company NAIC Number

If submitting more photographs than will fit on the preceding page, affix the additional photographs below. Identify all photographs with: date taken; "Front View" and "Rear View"; and, if required, "Right Side View" and "Left Side View." When applicable, photographs must show the foundation with representative examples of the flood openings or vents, as indicated in Section A8.

Photo Three

Photo Three Caption Right Side View

Clear Photo Three

Photo Four

Photo Four Caption Left Side View

Clear Photo Four