

Structure No.: 9999-999 Route: I ### Cycle No.: 17
Name: I ### over Roadway Insp. Date: 7/13/2012

LOAD RATING SUMMARY SHEET (LRSS)

(Form NJ-BI-101 Created 1/25/2011)

Project Information:

Group: STXX Agreement No.: 2011BI999Z Contract ID: 01-00001 Agree/Mod No.: 00

Rating Information:

Method: LRFR: Yes LFR: Yes ASR: No Other (Specify): _____

Rating Date: 8/31/2012 Computer Software Used: LARS Bridge V8i Version: 5.00.06.09

Load Testing: No Cycle when Rating Performed: 17 Design Load: HS 20

Structure Information:

Plans Available? Yes Contract Designation: I-999-99(99)99

Overlay? Yes Considered in Rating? Yes Type/Thickness: LMC/1.25"

Section Losses? No Considered in Rating? N/A Item 59: 7

For LRFR Use Only:

Dynamic Load Allowance: 1 Condition Factor: 1 System Factor: 1

ADTT (one direction): 8,050 Resistance Factor: LARS Calculated FCM: No

Load Rating Engineer (LRE):

Name: John A. Smith Firm: ABC Consultants Initial: JAS

Load Rating Reviewer (LRR) certification as per the NBIS Title 23 CFR Section 650.309(c):

Name: Jane B. Brown N.J. P.E. No.: 24GE00000001

Firm: ABC Consultants

I certify that this rating is an accurate representation of the subject structure, considering all deterioration and/or changes to loading conditions, to the extent determinable by research and visual inspection and testing performed. I am charged with the overall responsibility for bridge capacity evaluation for the above mentioned structure.

Sign and Seal if
Rating Performed
in this Cycle

Sign

Date

Structure No.: 9999-999 Route: I ### Cycle No.: 17
 Name: I ### over Roadway Insp. Date: 7/13/2012

LOAD RATING SUMMARY SHEET (LRSS) (cont.)

Rating Comments:

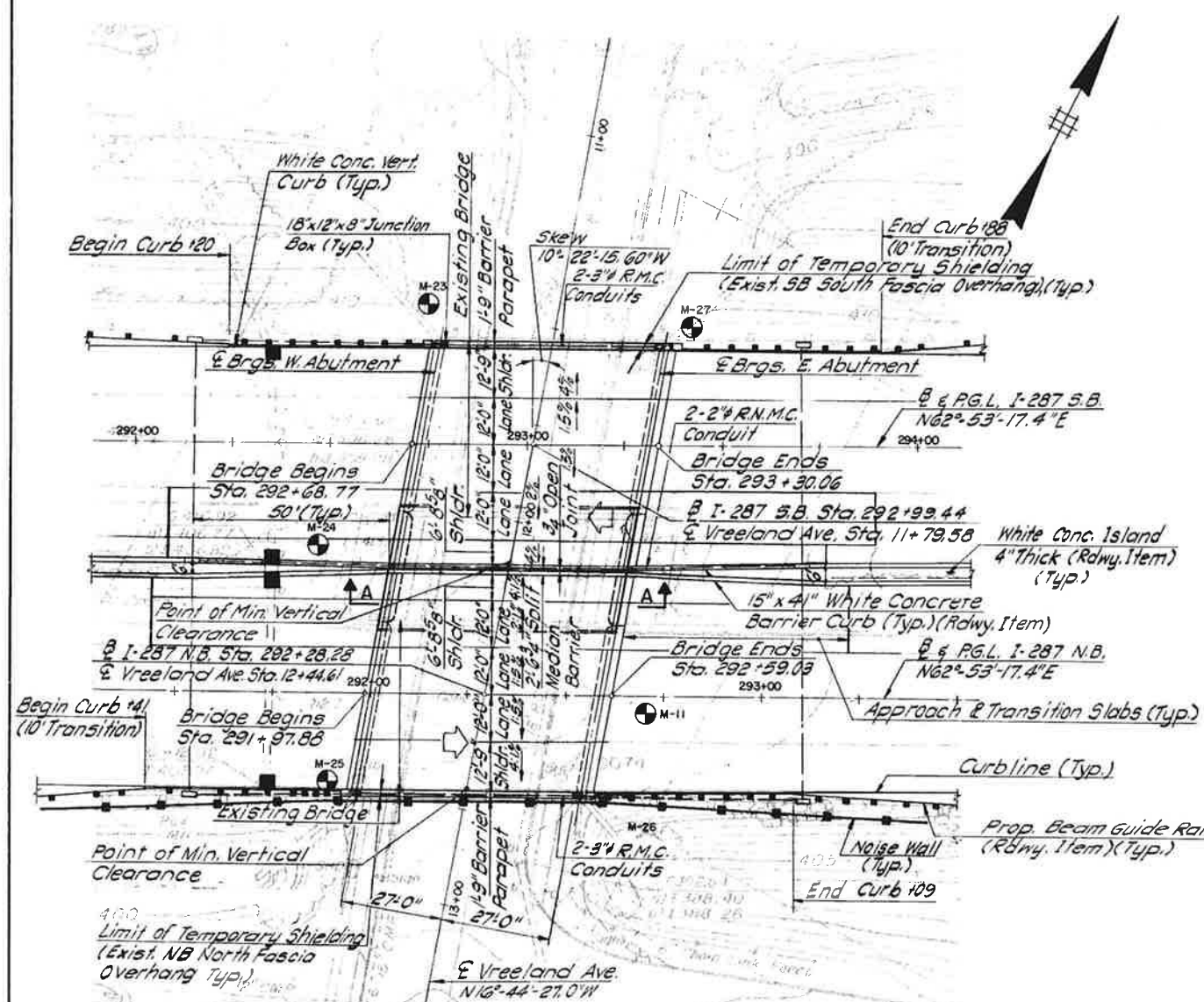
The Load Factor and LRFR ratings, computed in accordance with the FHWA directive dated November 1993, AASHTO Manual for Bridge Evaluation, 2011, as modified by the NJDOT Highway Bridge Load Rating Manual and Section 43 of the NJDOT Design Manual, Bridges and Structures, are as follows:

<u>Material</u>	<u>Compressive Strength f'_c</u>	<u>Tensile Strength</u>	<u>Allowable Stresses (Psi)</u>		
			<u>Yield</u>	<u>Inventory</u>	<u>Operating</u>
Concrete	3,000	---	---	1,200	1,650
Reinforcing Steel	---	---	40,000	20,000	28,000
Prestressed Concrete (Beam)	5,000	---	---	2,000	2,750
Prestressed Steel Strands (original)	---	250,000	---	175,000	---
Prestressed Steel Strands (widened)	---	270,000	---	---	---

			Rating (Tons) / Rating Factor							
			LFR				LRFR			
			As-Built		As-Insp.		As-Built		As-Insp.	
Member	Truck Type (Tons)		Inv.	Op.	Inv.	Op.	Inv.	Op. ¹	Inv.	Op. ¹
Interior Girder ² (Original Section) 01-IG012 Cond. Rating = 7	H15	(15T)	---	---	---	---	---	---	---	---
	HL-93	(NL)	---	---	---	---	1.19	1.54	1.19	1.54
	HS-20	(36T)	42	71	42	71	1.45	1.88	1.45	1.88
	3	(25T)	41	68	41	68	---	1.95	---	1.95
	3S2	(40T)	56	94	56	94	---	1.62	---	1.62
	3-3	(40T)	63	105	63	105	---	1.89	---	1.89
	SU4	(27T)	40	66	40	66	---	1.99	---	1.99
	SU5	(31T)	41	69	41	69	---	1.79	---	1.79
	SU6	(35T)	43	73	43	73	---	1.69	---	1.69
	SU7	(39T)	46	77	46	77	---	1.61	---	1.61

¹ Operating level rating of design load or legal load rating

² Controlling Rating
(NL) = Notional Load



- LEGEND:**
- 1959 Borings
 - Proposed 18"x36" Junction Box
 - Proposed Inlet

NOTE:
For Entrance Guide Rail Connection Detail, see Structure Plan and for Exit Guide Rail Connection (Type B) Detail, see Roadway Plan.

GENERAL NOTES:

DESIGN SPECIFICATIONS:
(A) 1989 AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES (WITH INTERIMS) AS MODIFIED BY SECTION 3 OF N.J.DOT DESIGN MANUAL FOR BRIDGES AND STRUCTURES.
(B) ALLOWABLE FATIGUE STRESSES BASED ON CASE 1 OF AASHTO TABLE 10.3.2A.

CONSTRUCTION SPECIFICATIONS:
1989 N.J.DOT STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION AS MODIFIED BY THE SUPPLEMENTARY SPECIFICATIONS.

LIVE LOAD: (NEW DECK AND GIRDERS)
(A) AASHTO HS20(MS18) + 25%(HS25) OR TANDEM 24 KIP AXLES AT 4 FOOT CENTERS, WHICHEVER GOVERNS.
(B) LIVE LOAD DEFLECTION SHALL NOT EXCEED 1/1000 OF THE SPAN LENGTH.

LIVE LOAD: (EXISTING DECK AND GIRDERS)
AASHTO HS20(MS18) OR TANDEM 24 KIP AXLES AT 4 FOOT CENTERS, WHICHEVER GOVERNS.

FUTURE DEAD LOADS:
NONE

CONCRETE DESIGN STRESSES:
(A) SPECIFIED DESIGN COMPRESSIVE STRENGTHS (F'_c)
CLASS A = 4,000 P.S.I.
CLASS B = 3,000 P.S.I.
(B) CLASS DESIGN STRENGTHS
CLASS A = 4,600 P.S.I.
CLASS B = 3,700 P.S.I.
(C) ALLOWABLE STRENGTHS, EXTREME FIBER IN COMPRESSION
CLASS A = 1,600 PSI
DECK SLABS = 1,400 PSI
CLASS B = 1,200 PSI

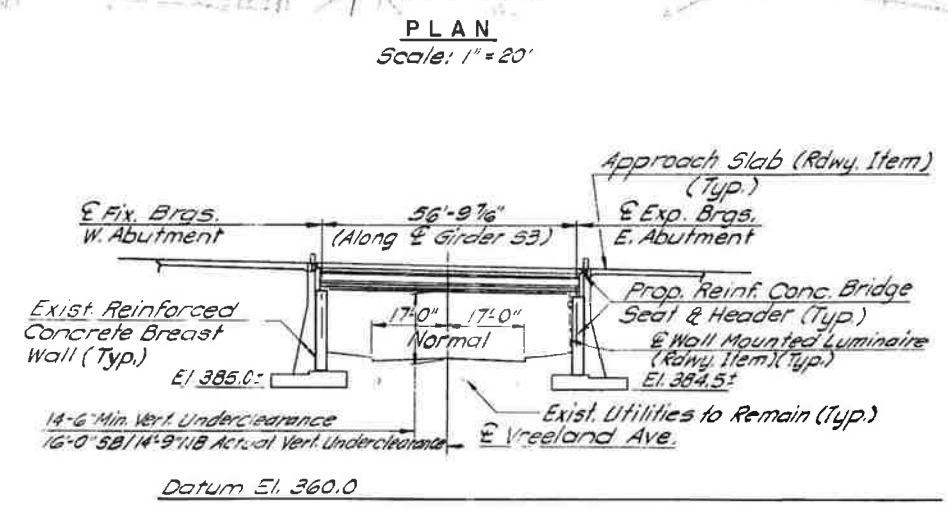
REINFORCEMENT STEEL:
(A) ASTM A615, GRADE 60.
TENSILE STRENGTH F_s = 24,000 P.S.I.

PRETENSIONED PRESTRESSED CONCRETE STRESSES: (PROPOSED GIRDERS)
MINIMUM STRENGTH AT 28 DAYS F'_c = 5,000 P.S.I.
MINIMUM STRENGTH AT TRANSFER F'_{ci} = 4,000 P.S.I.

PRESTRESSING STEEL: (PROPOSED GIRDERS)
ASTM A416 - 88b, 1/2" DIAMETER, TYPE 270K
ULTIMATE STRENGTH PER STRAND 41.3 KIPS
INITIAL TENSIONS PER STRAND 28.9 KIPS
STRESS RELIEVED STRANDS

SUPERSTRUCTURE:
COMPOSITE PRECAST PRESTRESSED CONCRETE I-BEAMS AND REINFORCED CONCRETE DECK SLAB
1-1/4" LATEX MODIFIED CONCRETE OVERLAY FOR PROPOSED DECK SLAB

EXISTING PLANS:
PLANS OF THE EXISTING STRUCTURES ARE AVAILABLE FOR INSPECTION AT THE OFFICE OF MR. DANIEL J. WOLFE, BUREAU OF STRUCTURAL ENGINEERING SERVICES, NEW JERSEY DEPARTMENT OF TRANSPORTATION, 1035 PARKWAY AVENUE, TRENTON, NEW JERSEY, 08625



SECTION A-A
Scale: 1" = 20'



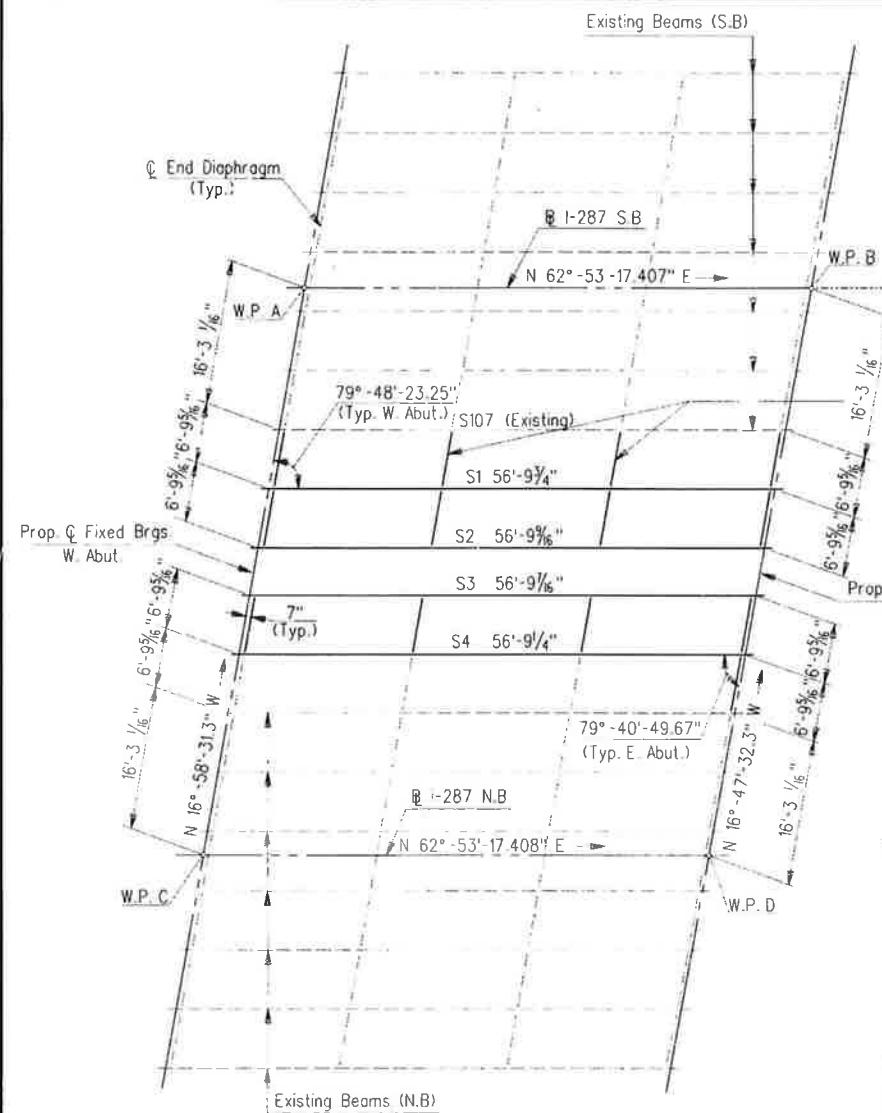
REVISION	BY	C'D	DATE

BRIDGE NO. 10 & 11

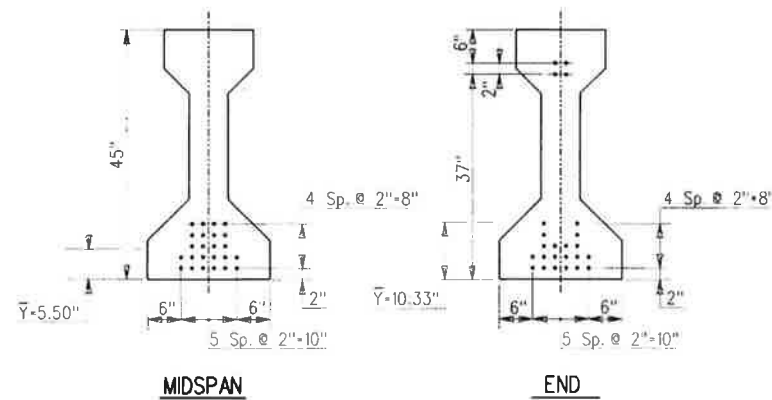
NEW JERSEY DEPARTMENT OF TRANSPORTATION

GENERAL PLAN AND ELEVATION

SAMPLE

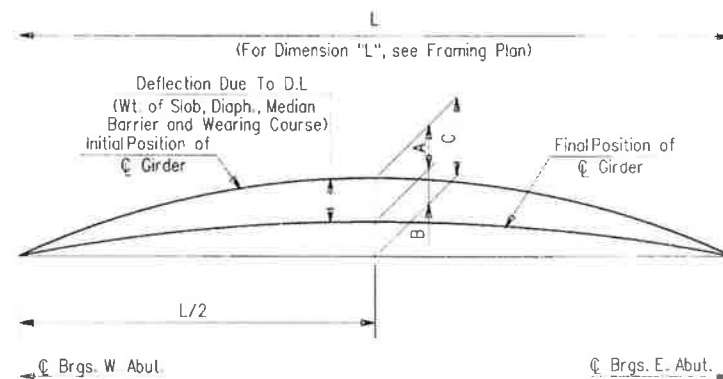


FRAMING PLAN
Scale: 1"=10'-0"



STRAND PATTERNS - 24 STRANDS
Scale: 3/4"=1'-0"

ESTIMATE OF QUANTITIES			
ITEM	UNIT	QUANTITY	AS-BUILT QUANTITY
PRESTRESSED CONCRETE BEAMS, 45"	L.F.	233	233
STRUCTURAL STEEL BEARINGS FOR PRESTRESSED CONCRETE BEAMS (APPROX. 3112 LBS.)	L.S.	1	1



CAMBER DIAGRAM
N.T.S.

CAMBER NOTES:

1. Initial Girder position is based upon a Camber Growth Factor of 1.8 at time of erection, which is anticipated to be approximately 30 days after fabrication.
2. The Camber Diagram should be adjusted if a different Camber Growth Factor or time of erection is anticipated.

REFERENCES:

1. For Working Point Layout, see Sheet No. B-170
2. For Girder Details, see Sheet No. B-178
3. For Existing Girder Details, see Existing Plans.

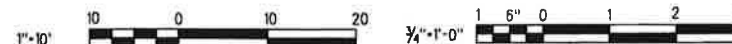
CAMBER TABLE			
GIRDER	A	B	C
S2 & S3	0.081	0.056	0.115
S1 & S4	0.088	0.049	0.115

All Cambers in Decimals of a Foot

NEW JERSEY DEPARTMENT OF TRANSPORTATION

FRAMING PLAN
AND GIRDER DETAILS

SAMPLE

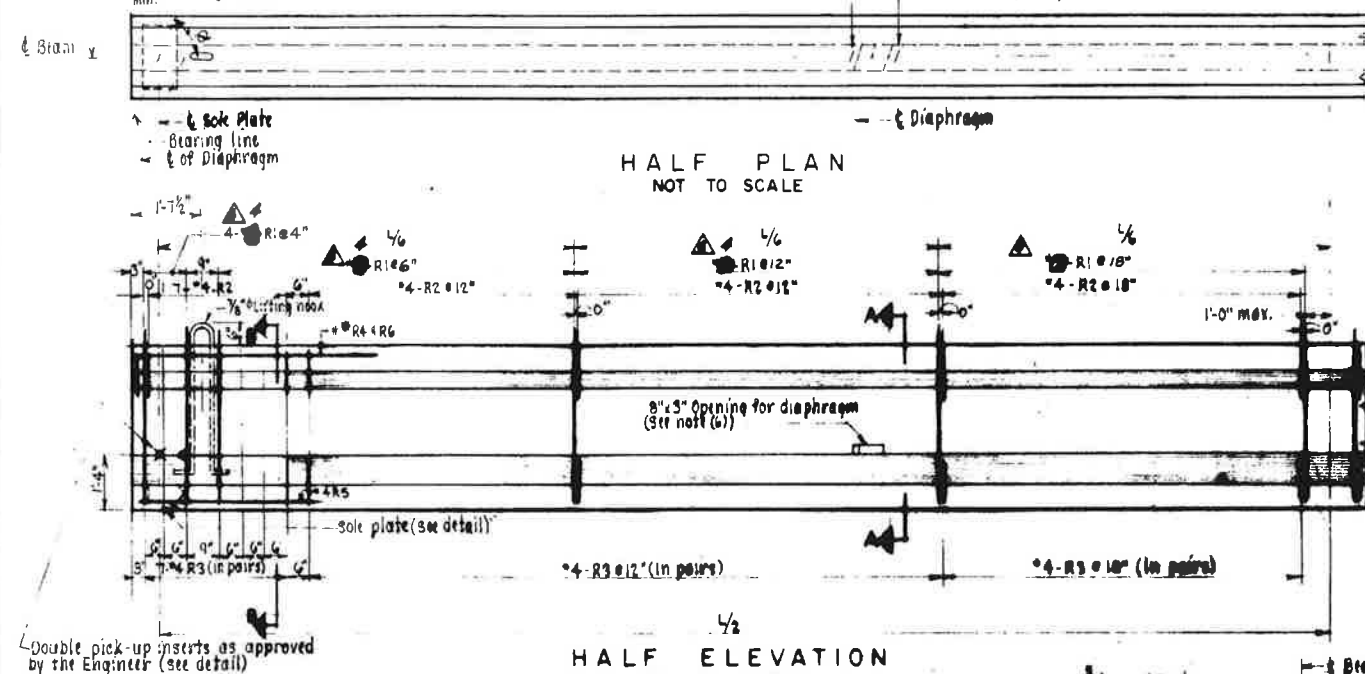


REVISION	BY	CKD.	DATE

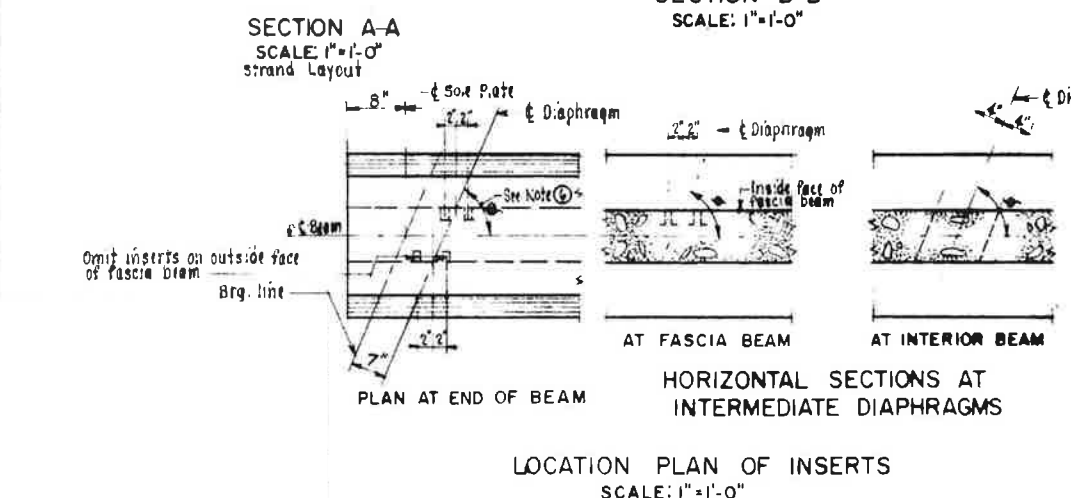
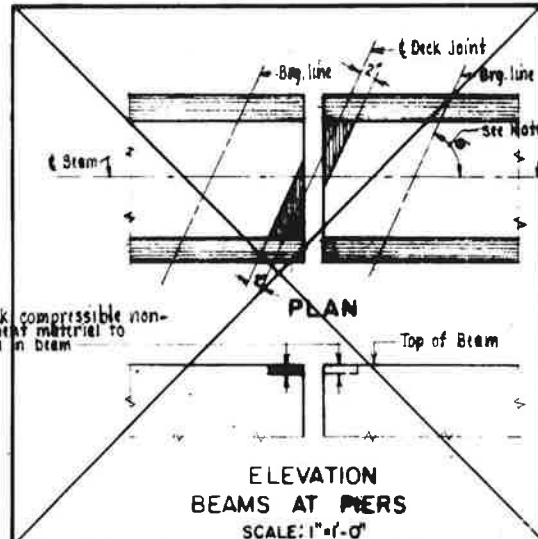
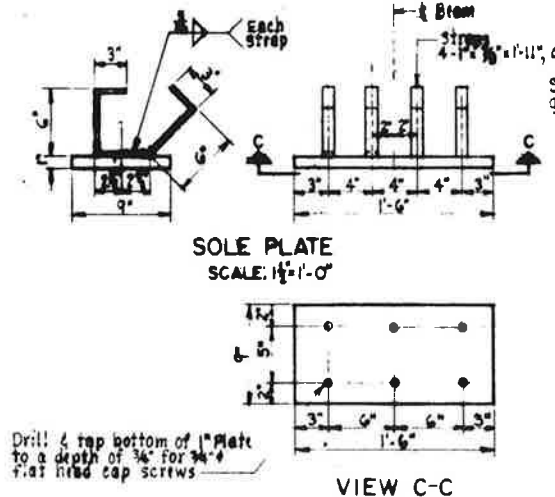
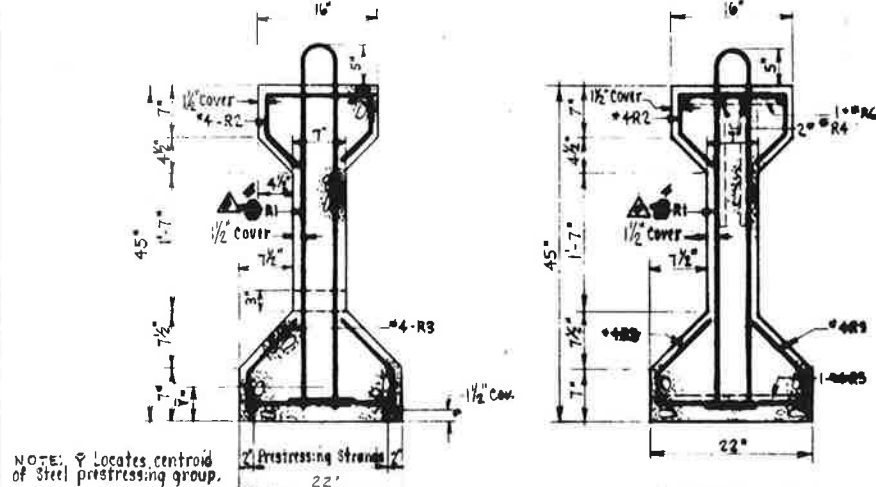
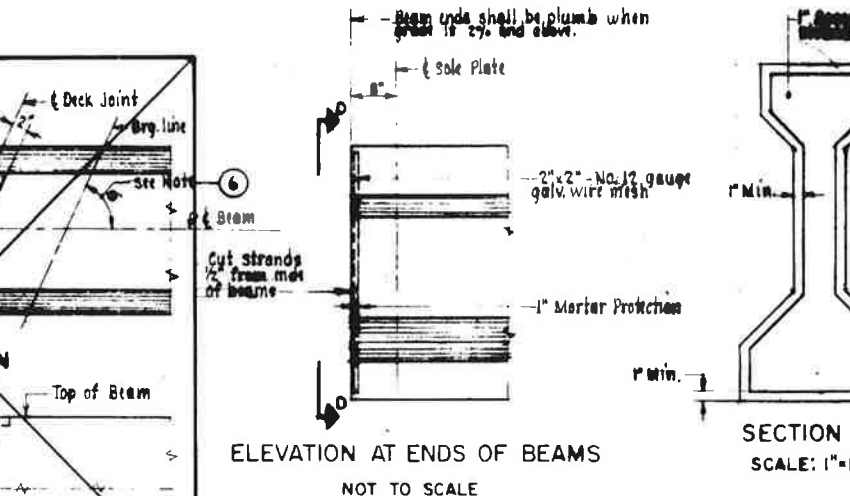
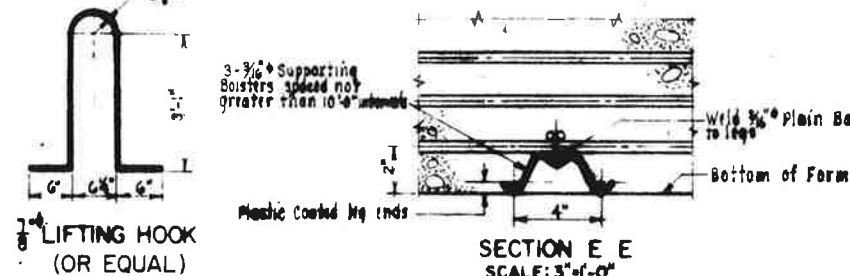
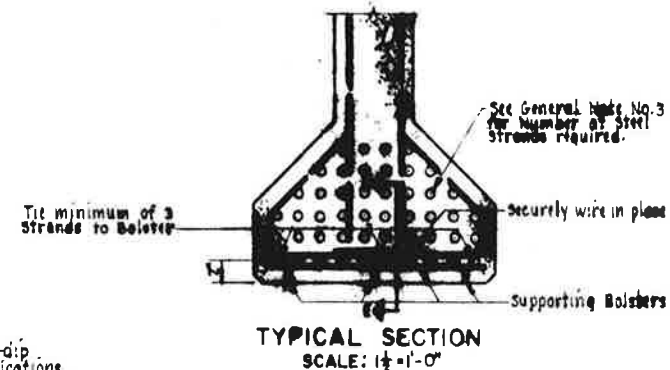
Interior Beam 8"
Fascia Beam 8"
Min. - See Note ⑥

Stirrups adjacent to 8"x3" opening shall be skewed

Beam Symmetrical about centerline



SCHEDULE OF MILD STEEL REINFORCEMENT									
MARK	SIZE	LENGTH	TYPE	A	B	C	D		
R1	#4	8'-3"	1	4'-0 1/2"	4'				
R2	#4	3'-3"	3	5 3/8"	5 3/8"	5"	1'-1"		
R3	#4	2'-6"	4	7 3/4"	7 3/4"	4 1/4"	1'-3"		
(D) R4	#5	8'-0"	2	6'-6"	1'-6"				
R5	#4	11'-0"	2	4'-9"	1'-6"	4'-9"			
(D) R6	#5	14'-0"	2	6'-6"	1'-0"	6'-6"			



GENERAL NOTES

- DESIGN SPECIFICATIONS
A.A.S.H.T.O. Standard Specifications for Highway Bridges, 1989
- DESIGN LOADS
A.A.S.H.T.O. HS20-44+10%
- PRESTRESSING STEEL
The prestressing strands shall be 1/2" x 7-Wire Uncoated Steel Strands. Each strand shall be given an initial tension of 29,000 lbs. Any change in the system of prestressing must be accompanied by complete calculations for approval by the Engineer.
- CONCRETE
All exposed corners shall be chamfered 1/4" or rounded to 3/4" radius. Angles of intersection between webs and flanges shall be rounded to not less than 1/4" radius. Top surface of beams shall be roughened to satisfaction of Engineer. At approximate time of initial set all laitance shall be removed with a stiff wire brush.
- SOLE PLATES
Cost of sole plates shall be included in price bid for Prestressed Concrete Beams.
- DIAPHRAGMS
For show each and location of diaphragms see Superstructure sheets.
- MILD STEEL REINFORCEMENT
Reinforcement bars shall conform to A.S.T.M. A601 and shall have 100% yield point. Cost of furnishing and placing reinforcement steel shall be included in the price bid for Prestressed Concrete Beams.

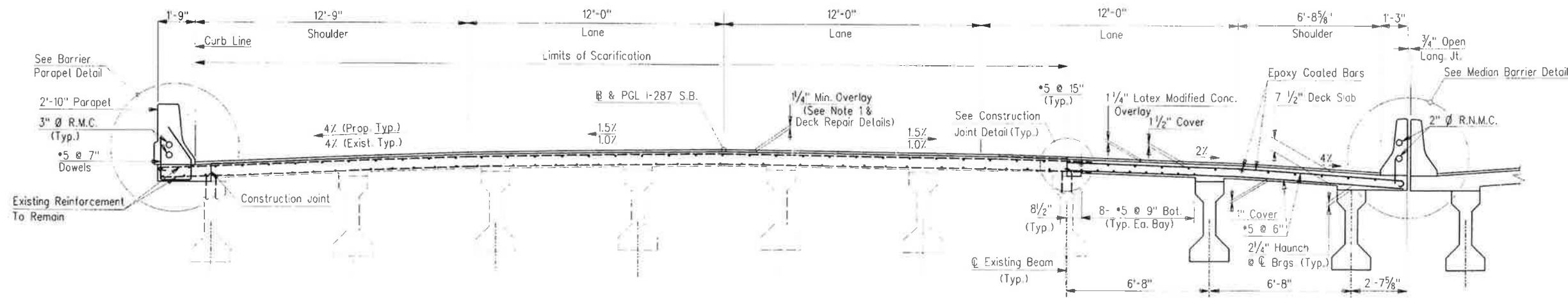
PLATE 34-1

NEW JERSEY DEPARTMENT OF TRANSPORTATION
BRIDGES AND STRUCTURES DESIGN

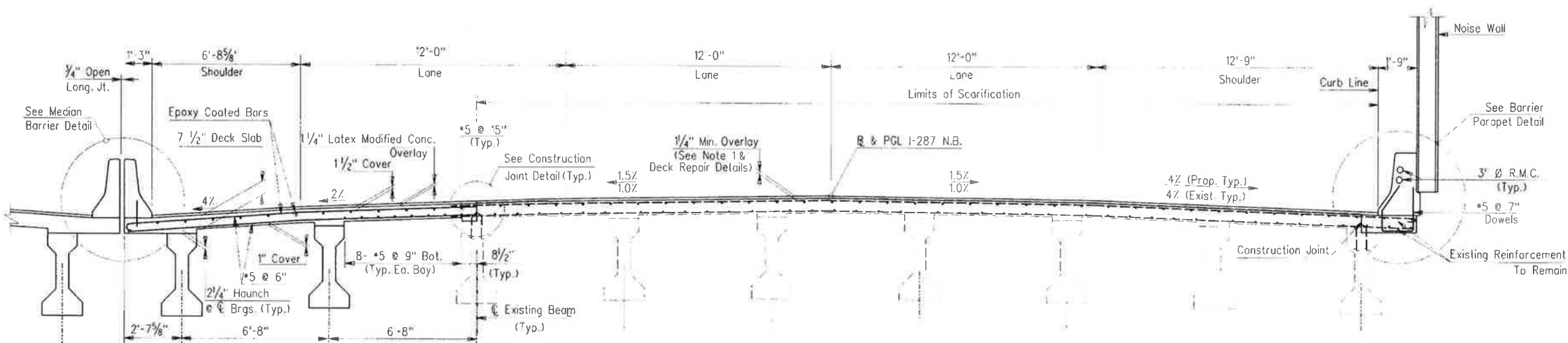
SAMPLE

45" PRETENSIONED PRESTRESSED CONCRETE BEAMS

SAMPLE



TYPICAL CROSS SECTION-SOUTHBOUND
Scale: 3/8" = 1'-0"



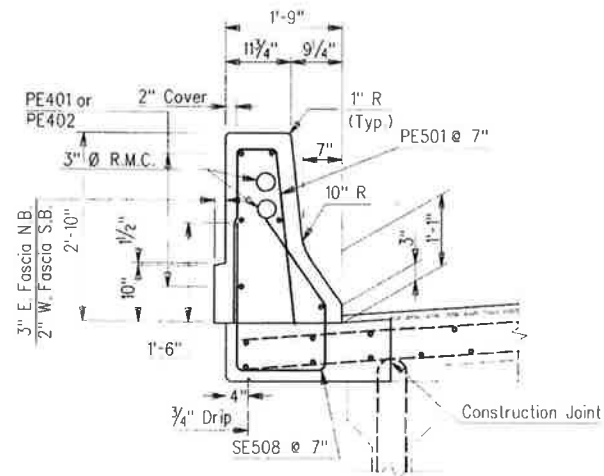
TYPICAL CROSS SECTION-NORTHBOUND
Scale: 3/8" = 1'-0"

NOTES:

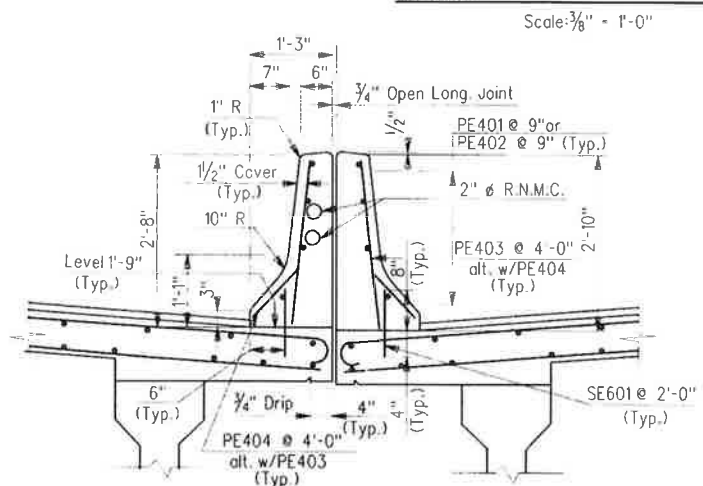
1. Contractor Shall Scarify Existing Deck as required to provide 1/4" Min. Latex Modified Concrete Overlay for Final Grade and Cross Slopes, while maintaining the required cover. In any case, the Scarification depth shall be a minimum of 1/4".
2. See Standard and Supplementary Specifications for cleaning contact surface and placing L.M.C. Overlay.
3. Epoxy Bonding Coat required for Joint in First-Course Slab. Excess overflow at top to be removed by sandblasting or high pressure water blasting.
4. Reinforcing steel in Parapets and in top layer of deck shall be Epoxy Coated.

REFERENCES:

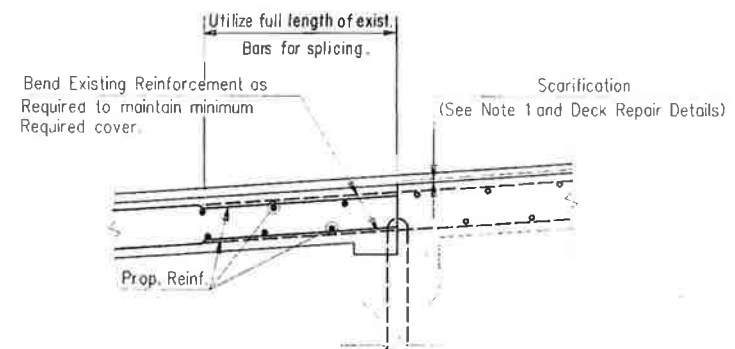
1. For Existing Deck Repair & Scarification details, see Sheet B-163
2. For Deck Joint Details, see Sheet No. B-162



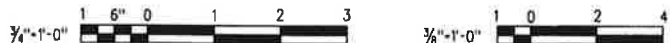
BARRIER PARAPET DETAIL
Scale: 3/4" = 1'-0"



MEDIAN BARRIER DETAIL
Scale: 3/4" = 1'-0"



CONSTRUCTION JOINT DETAIL
Scale: 1" = 1'-0"

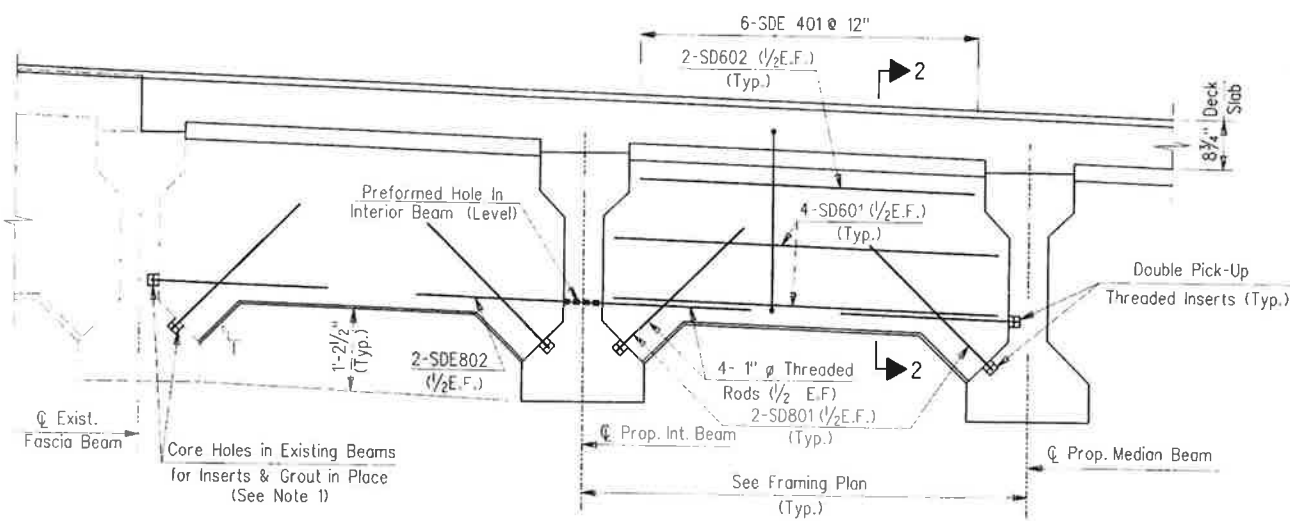


REVISION	BY	CHKD.	DATE

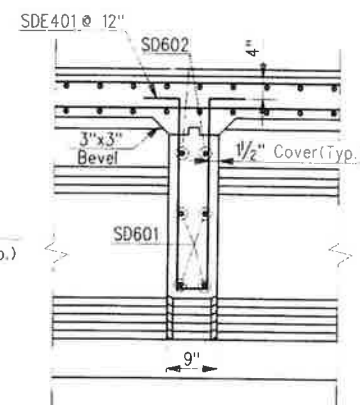
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SUPERSTRUCTURE CROSS SECTION

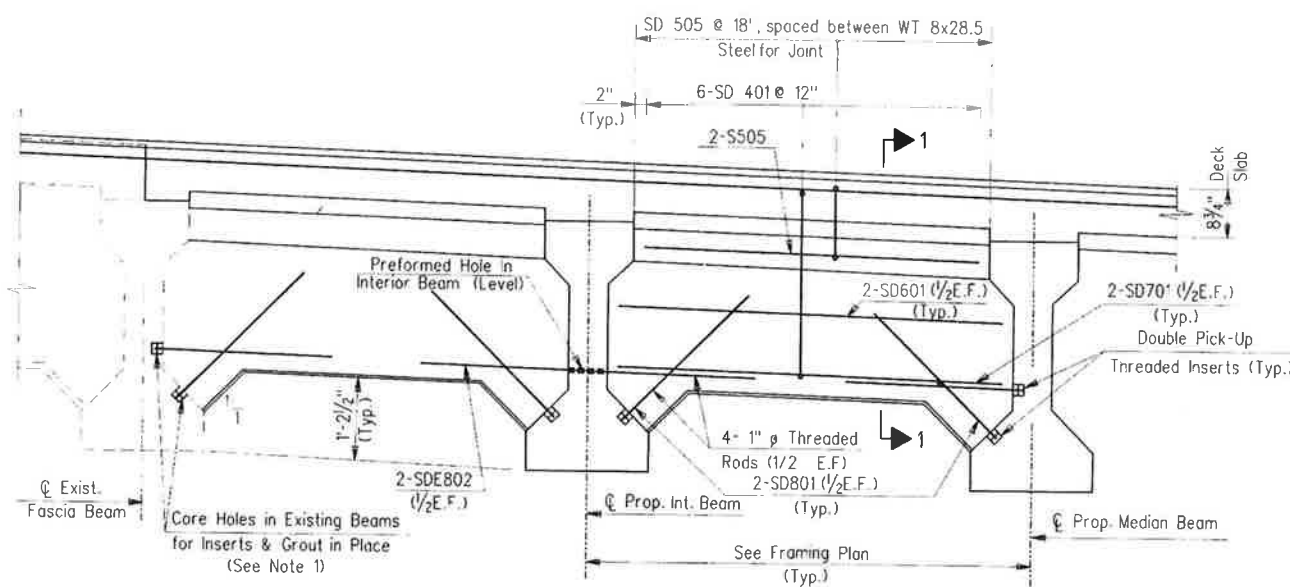
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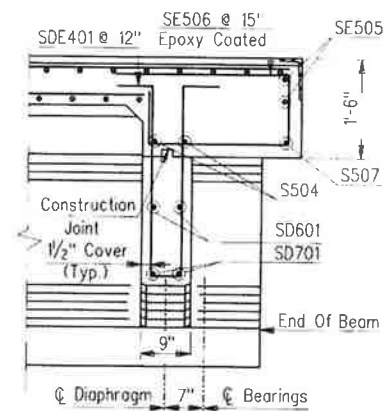
INTERMEDIATE DIAPHRAGMS



SECTION 2-2



END DIAPHRAGMS



SECTION 1-1

NOTES:

1. Before coring holes into Existing Beams, a Pachometer shall be used to locate existing reinforcement.
2. Top of Girder shall be clean, free of laitance, and intentionally roughened to a full amplitude of approx. 1/4".
3. Diaphragm Concrete is Paid under "Concrete in Superstructure, Deck Slabs (Class A)."

REFERENCES:

1. For Deck Plan, see Sheet No. B-179
2. For Bar List, see Sheet No. B-188

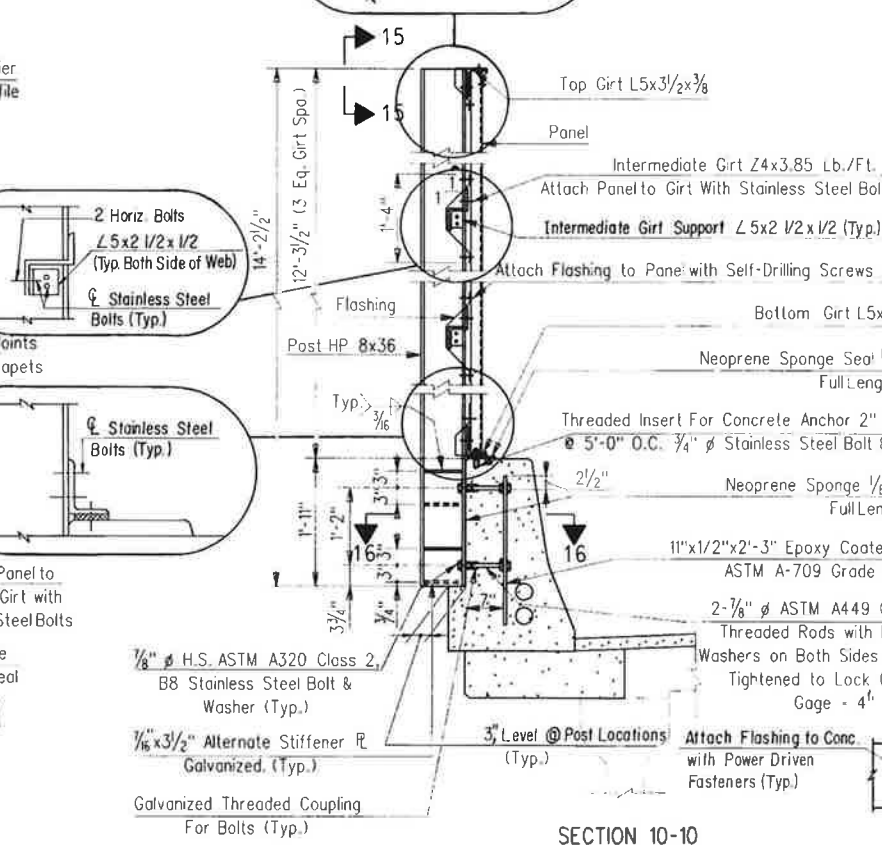
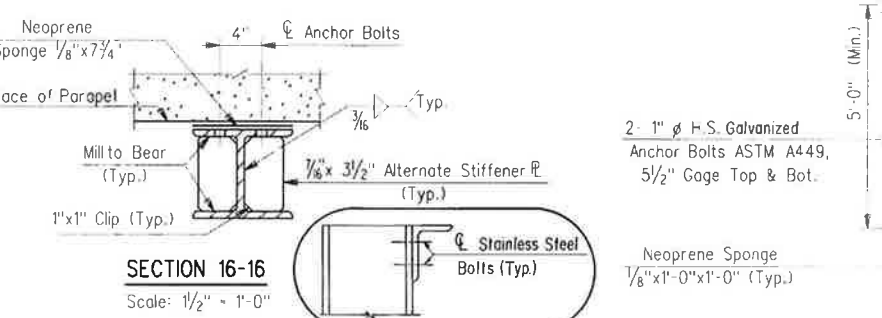
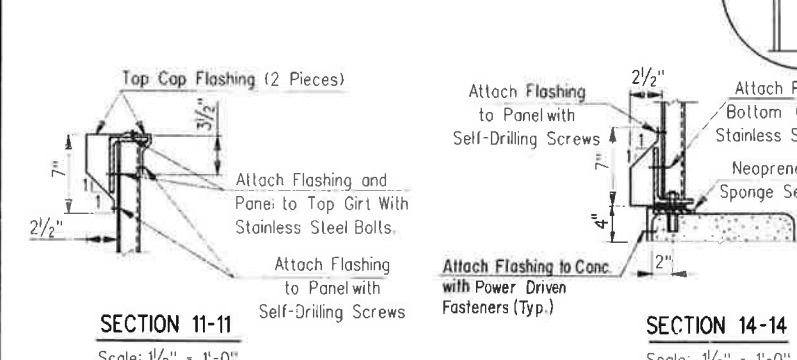
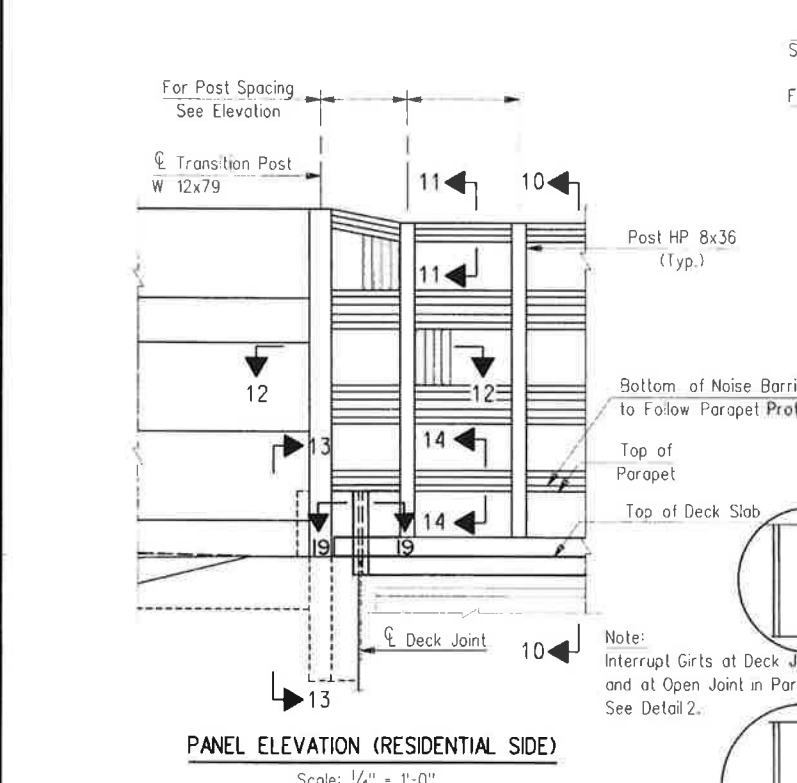
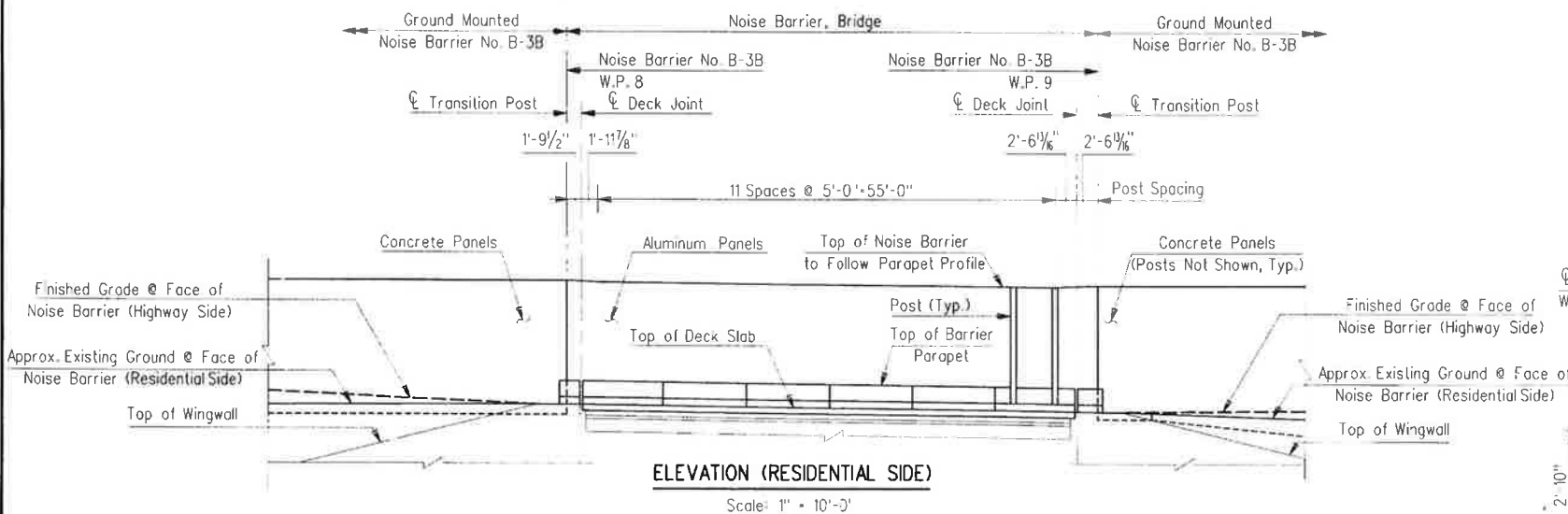
NEW JERSEY DEPARTMENT OF TRANSPORTATION

DIAPHRAGM DETAILS

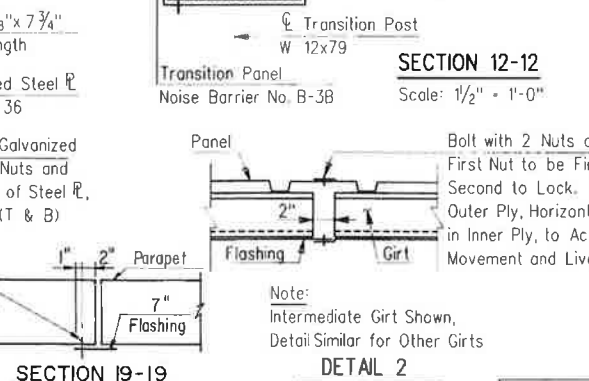
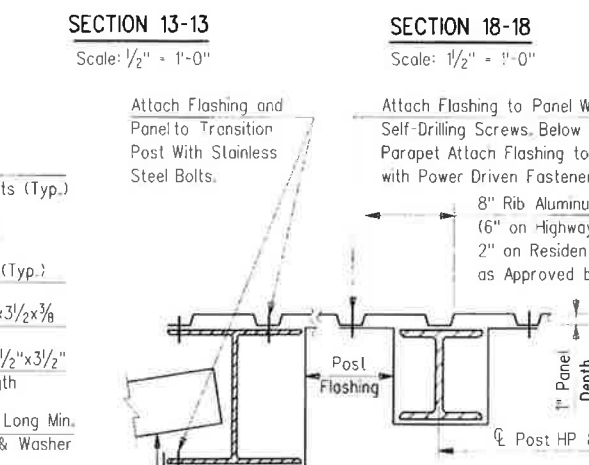
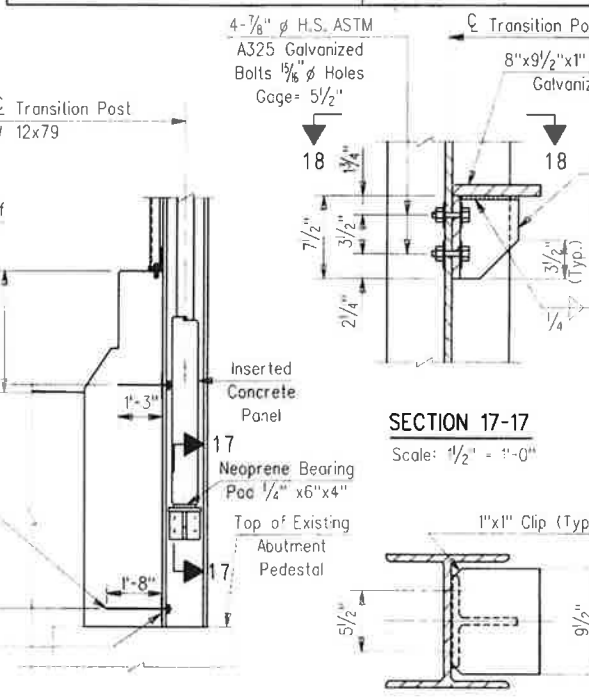
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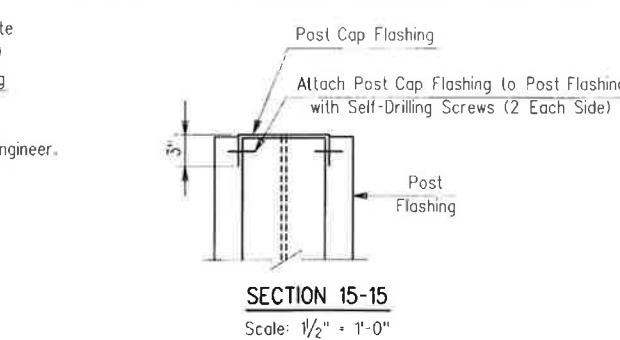
REVISION	BY	CHKD.	DATE



ESTIMATE OF QUANTITIES			
ITEM	UNIT	QUANTITY	AS-BUILT QUANTITY
NOISE BARRIER, BRIDGE	SF	786	



- NOTES:**
- DESIGN CRITERIA:**
- DESIGN WIND LOAD IS 50 POUNDS PER SQUARE FOOT, WHICH CORRESPONDS TO A WIND VELOCITY OF 100 MILES PER HOUR.
- MATERIALS:**
- STEEL POSTS SHALL CONFORM TO ASTM A709, GRADE 36, AND SHALL BE GALVANIZED AFTER FABRICATION.
 - GIRTS AND CONNECTION ANGLES SHALL BE ALUMINUM ALLOY 6061-T6 CONFORMING TO ASTM B308.
 - PANELS SHALL BE ALUMINUM ALLOY ALCLAD 3004-H16, WITH A BARE MATERIAL THICKNESS OF 0.04 IN.
 - FLASHING SHALL BE FLAT SHEET ALUMINUM ALLOY ALCLAD 3004 CONFORMING TO ASTM B209, WITH A BARE MATERIAL THICKNESS OF 0.04 IN.
 - BOLTS AND SHEET FASTENERS SHALL BE STAINLESS STEEL OF SIZES INDICATED AND IN ACCORDANCE WITH SIDING MANUFACTURERS STANDARDS.
- CONSTRUCTION:**
- CONTRACTOR SHALL SUBMIT DETAILED SHOP DRAWINGS FOR THE ENGINEER'S APPROVAL.
 - NOISE BARRIER SHALL BE ERECTED AND CONSTRUCTED SO THAT IT WILL NOT PERMIT THE PASSAGE OF LIGHT.
 - POSTS SHALL BE ERECTED PERFECTLY PLUMB.
 - ALL GIRTS SHALL FOLLOW THE CONCRETE PARAPET VERTICAL PROFILE. GIRTS SHALL HAVE SLOTTED HOLES TO ALLOW FOR VERTICAL AND HORIZONTAL ADJUSTMENT DUE TO VERTICAL PROFILE.
 - PANEL LENGTH SHALL BE FIELD CUT TO PROVIDE PROPER LAP FOR CONNECTION AT TOP AND BOTTOM GIRTS DUE TO PARAPET VERTICAL PROFILE.
 - NOISE BARRIER FINISH COLOR SHALL BE BROWN AND SHALL MATCH THE FINISH COLOR OF THE ADJACENT GROUND MOUNTED NOISE BARRIERS.



- REFERENCES:**
- For Parapet Joint Spacing, see Sheet No. B-179.
 - For Working Point Locations and Noise Barrier No. B-3B, see Sheet No. B-246.
 - For Details of Existing Structure, see Existing Plans.

NEW JERSEY DEPARTMENT OF TRANSPORTATION

NOISE BARRIER
ELEVATION & DETAILS

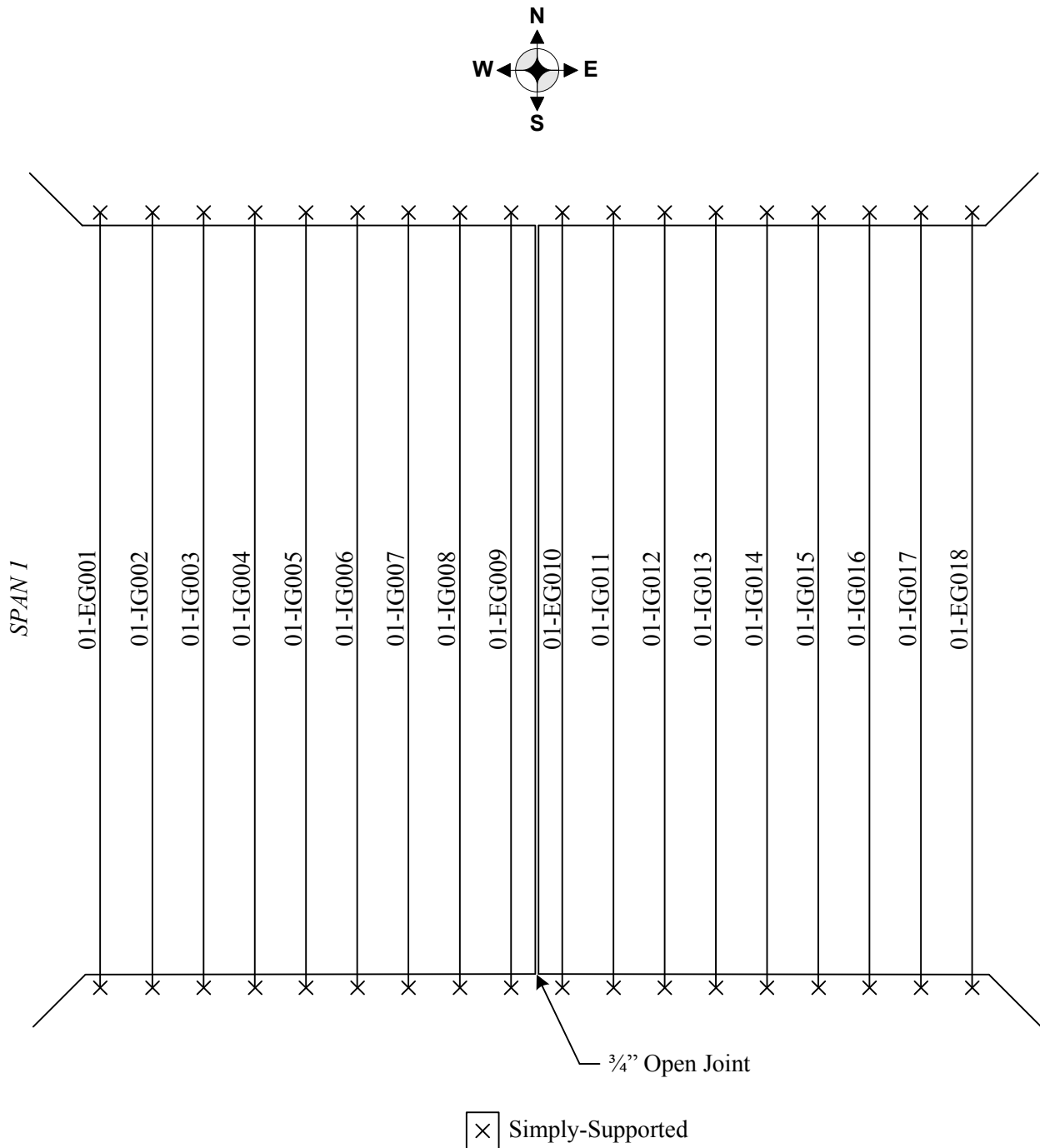
SAMPLE

REVISION	BY	CHKD.	DATE

Structure No.: 9999-999 Route: I ### Cycle No.: 17
 Name: I ### over Roadway Insp. Date: 7/13/2012

LARS Member Description Sketch:

I ### OVER ROADWAY



Structure No.: 9999-999	Route: I ###	Cycle No.: 17
Name: I ### over Roadway		Insp. Date: 7/13/2012

CALCULATIONS – DEAD LOAD:

DEAD LOADS						
Center-Center Bearing	56.786	ft				
Beam Spacing	6.667	ft				
Overhang Spacing	2.667	ft				
Skew	10.194	degrees				
Beam Top Width	1.333	ft				
Number of Beams	18					
DIAPHRAGMS (Continuous-Constant Stage 1)						
	Diaphragms (Concrete)					
	Length	6.774	ft			
	Width	0.750	ft			
	Height	3.167	ft			
	Volume	16.087	cf			
	Total Weight	2413.089	lb			
	Weight per LF	42.494	plf			
	Interior Beam (4 total)	169.976	plf			
	Exterior Beam (4 total)	84.988	plf			
SIP FORMS (Continuous-Constant Stage 1)						
Interior Beams 7 & 12	Weight per LF	40.000	plf			
Interior Beams 8 & 11	Weight per LF	80.000	plf			
Exterior Beams 9 & 10	Weight per LF	40.000	plf			
PARAPET (Continuous-Constant)						
	Fascia Barrier			Median Barrier		
	W1	0.979	ft	W1	0.500	ft
	W2	0.188	ft	W2	0.167	ft
	W3	0.583	ft	W3	0.583	ft
	H1	0.250	ft	H1	0.250	ft
	H2	0.833	ft	H2	0.833	ft
	H3	1.750	ft	H3	1.583	ft
	Area (x 2)	7.061	sf	Area (x 2)	4.069	sf
	Weight per LF	1059.115	plf	Weight per LF	610.417	plf
	Noise Wall	187.677	plf (beams 10-18 only)	Weight per beam	33.912	plf
	Weight per beam (1 - 9)	58.840	plf			
	Weight per beam (10 - 18)	79.693	plf			
LMC OVERLAY (Continuous-Constant)						
	Height of LMC	0.104	ft			
	Interior Beam Weight per LF	104.167	plf			
	Exterior Beam Weight per LF	93.750	plf			
TOTAL DEAD LOAD						
	Beam 1 Continuous-Constant Stage 1	84.988				
	Beams 2 - 6 Continuous-Constant Stage 1	169.976				
	Beam 7 Continuous-Constant Stage 1	209.976				
	Beam 8 Continuous-Constant Stage 1	249.976				
	Beam 9 Continuous-Constant Stage 1	124.988				
	Beam 10 Continuous-Constant Stage 1	124.988				
	Beam 11 Continuous-Constant Stage 1	249.976				
	Beam 12 Continuous-Constant Stage 1	209.976				
	Beams 13 - 17 Continuous-Constant Stage 1	169.976				
	Beam 18 Continuous-Constant Stage 1	84.988				
	Beam 1 Continuous-Constant	186.502				
	Beams 2 - 8 Continuous-Constant	196.918				
	Beam 9 Continuous-Constant	186.502				
	Beam 10 Continuous-Constant	207.355				
	Beams 11 - 17 Continuous-Constant	217.771				
	Beam 18 Continuous-Constant	207.355				

Structure No.: 9999-999 Route: I ### Cycle No.: 17
 Name: I ### over Roadway Insp. Date: 7/13/2012

CALCULATIONS - LARS MEMBER SUMMARY (TABULAR) REPORT:

LOAD ANALYSIS AND RATING SYSTEM -- ASD/LFD/LRFD v5.00.06.09 PAGE 1

BRIDGE / MEMBER DATA

SUMMARY REPORT

Bridge ID: 9999999_20120831cy17_01of01
 Bridge Name I ### over Roadway
 Path Name T:\
 File Name 9999999_20120831cy17_01of01.bmd

Comments:

Member Descr: 01-EG001

Mem ID	Truck	Rating (Tons)				Rating (Tons)				Rating Factor					
		ASD		LFD		LRFD		LRFD		LRFD		LRFD		LRFD	
		INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	LGL	Contr Type
G01	HL93	88	V	147	V	88	V	147	V	2.00	V	2.59	V		
G01	HS20	88	V	147	V	88	V	147	V	2.43	V	3.15	V		
G01	3	85	V	141	V	85	V	141	V	3.36	V	4.36	V	3.27	V
G01	3S2	114	V	190	V	114	V	190	V	2.79	V	3.62	V	2.71	V
G01	3-3	131	V	219	V	131	V	219	V	3.25	V	4.22	V	3.16	V
G01	SU4	83	V	138	V	83	V	138	V	3.05	V	3.95	V	3.33	V
G01	SU5	85	V	143	V	85	V	143	V	2.74	V	3.55	V	3.00	V
G01	SU6	90	V	151	V	90	V	151	V	2.59	V	3.35	V	2.83	V
G01	SU7	96	V	160	V	96	V	160	V	2.46	V	3.19	V	2.69	V

Member Descr: 01-IG002

Mem ID	Truck	Rating (Tons)				Rating (Tons)				Rating Factor					
		ASD		LFD		LRFD		LRFD		LRFD		LRFD		LRFD	
		INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	LGL	Contr Type
G02	HL93	43	V	72	V	43	V	72	V	1.21	V	1.57	V		
G02	HS20	43	V	72	V	43	V	72	V	1.47	V	1.90	V		
G02	3	42	V	69	V	42	V	69	V	2.03	V	2.63	V	1.97	V
G02	3S2	57	V	95	V	57	V	95	V	1.68	V	2.18	V	1.64	V
G02	3-3	65	V	108	V	65	V	108	V	1.96	V	2.55	V	1.91	V
G02	SU4	41	V	68	V	41	V	68	V	1.84	V	2.38	V	2.01	V
G02	SU5	42	V	70	V	42	V	70	V	1.65	V	2.14	V	1.81	V
G02	SU6	44	V	74	V	44	V	74	V	1.56	V	2.03	V	1.71	V
G02	SU7	47	V	79	V	47	V	79	V	1.49	V	1.93	V	1.63	V

Member Descr: 01-IG003

Mem ID	Truck	Rating (Tons)				Rating (Tons)				Rating Factor					
		ASD		LFD		LRFD		LRFD		LRFD		LRFD		LRFD	
		INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	LGL	Contr Type
G03	HL93	43	V	72	V	43	V	72	V	1.21	V	1.57	V		
G03	HS20	43	V	72	V	43	V	72	V	1.47	V	1.90	V		
G03	3	42	V	69	V	42	V	69	V	2.03	V	2.63	V	1.97	V
G03	3S2	57	V	95	V	57	V	95	V	1.68	V	2.18	V	1.64	V
G03	3-3	65	V	108	V	65	V	108	V	1.96	V	2.55	V	1.91	V
G03	SU4	41	V	68	V	41	V	68	V	1.84	V	2.38	V	2.01	V

Structure No.:	9999-999	Route:	I ###	Cycle No.:	17
Name:	I ### over Roadway			Insp. Date:	7/13/2012

G03	SU5	42	V	70	V	42	V	70	V	1.65	V	2.14	V	1.81	V
G03	SU6	44	V	74	V	44	V	74	V	1.56	V	2.03	V	1.71	V
G03	SU7	47	V	79	V	47	V	79	V	1.49	V	1.93	V	1.63	V

Member Descr: 01-IG004

Mem ID	Truck	Rating (Tons)				Rating (Tons)				Rating Factor					
		ASD		LFD		LRFD		LRFD		LRFD		LRFD		LRFD	
		INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	LGL	Contr Type
G04	HL93	43	V	72	V	43	V	72	V	1.21	V	1.57	V		
G04	HS20	43	V	72	V	43	V	72	V	1.47	V	1.90	V		
G04	3	42	V	69	V	42	V	69	V	2.03	V	2.63	V	1.97	V
G04	3S2	57	V	95	V	57	V	95	V	1.68	V	2.18	V	1.64	V
G04	3-3	65	V	108	V	65	V	108	V	1.96	V	2.55	V	1.91	V
G04	SU4	41	V	68	V	41	V	68	V	1.84	V	2.38	V	2.01	V
G04	SU5	42	V	70	V	42	V	70	V	1.65	V	2.14	V	1.81	V
G04	SU6	44	V	74	V	44	V	74	V	1.56	V	2.03	V	1.71	V
G04	SU7	47	V	79	V	47	V	79	V	1.49	V	1.93	V	1.63	V

Member Descr: 01-IG005

Mem ID	Truck	Rating (Tons)				Rating (Tons)				Rating Factor					
		ASD		LFD		LRFD		LRFD		LRFD		LRFD		LRFD	
		INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	LGL	Contr Type
G05	HL93	43	V	72	V	43	V	72	V	1.21	V	1.57	V		
G05	HS20	43	V	72	V	43	V	72	V	1.47	V	1.91	V		
G05	3	42	V	70	V	42	V	70	V	2.03	V	2.63	V	1.98	V
G05	3S2	57	V	95	V	57	V	95	V	1.68	V	2.18	V	1.64	V
G05	3-3	65	V	108	V	65	V	108	V	1.97	V	2.55	V	1.91	V
G05	SU4	41	V	68	V	41	V	68	V	1.84	V	2.39	V	2.01	V
G05	SU5	42	V	70	V	42	V	70	V	1.66	V	2.15	V	1.81	V
G05	SU6	44	V	74	V	44	V	74	V	1.56	V	2.03	V	1.71	V
G05	SU7	47	V	79	V	47	V	79	V	1.49	V	1.93	V	1.63	V

Member Descr: 01-IG006

Mem ID	Truck	Rating (Tons)				Rating (Tons)				Rating Factor					
		ASD		LFD		LRFD		LRFD		LRFD		LRFD		LRFD	
		INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	LGL	Contr Type
G06	HL93	43	V	72	V	43	V	72	V	1.21	V	1.57	V		
G06	HS20	43	V	72	V	43	V	72	V	1.47	V	1.90	V		
G06	3	42	V	69	V	42	V	69	V	2.03	V	2.63	V	1.97	V
G06	3S2	57	V	95	V	57	V	95	V	1.68	V	2.18	V	1.64	V
G06	3-3	65	V	108	V	65	V	108	V	1.96	V	2.55	V	1.91	V
G06	SU4	41	V	68	V	41	V	68	V	1.84	V	2.38	V	2.01	V
G06	SU5	42	V	70	V	42	V	70	V	1.65	V	2.14	V	1.81	V
G06	SU6	44	V	74	V	44	V	74	V	1.56	V	2.03	V	1.71	V
G06	SU7	47	V	79	V	47	V	79	V	1.49	V	1.93	V	1.63	V

Member Descr: 01-IG007

Mem ID	Truck	Rating (Tons)				Rating (Tons)				Rating Factor					
		ASD		LFD		LRFD		LRFD		LRFD		LRFD		LRFD	
		INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	LGL	Contr Type
G07	HL93	43	V	71	V	43	V	71	V	1.20	V	1.56	V		

Structure No.:	9999-999	Route:	I ###	Cycle No.:	17
Name:	I ### over Roadway			Insp. Date:	7/13/2012

G07	HS20	43	V	71	V	43	V	71	V	1.47	V	1.90	V
G07	3	41	V	69	V	41	V	69	V	2.02	V	2.62	V
G07	3S2	56	V	94	V	56	V	94	V	1.68	V	2.18	V
G07	3-3	64	V	106	V	64	V	106	V	1.96	V	2.54	V
G07	SU4	40	V	67	V	40	V	67	V	1.83	V	2.38	V
G07	SU5	41	V	69	V	41	V	69	V	1.65	V	2.14	V
G07	SU6	44	V	73	V	44	V	73	V	1.56	V	2.02	V
G07	SU7	47	V	78	V	47	V	78	V	1.48	V	1.92	V

Member Descr: 01-IG008

Mem ID	Truck	Rating (Tons)				Rating (Tons)				Rating Factor					
		ASD		LFD		ASD		LFD		LRFD		LRFD		LGL	
		INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	LGL	Contr Type
G08	HL93	45	V	75	V	45	V	75	V	1.45	V	1.82	V		
G08	HS20	45	V	75	V	45	V	75	V	1.69	V	2.12	V		
G08	3	43	V	72	V	43	V	72	V	2.18	V	2.72	V	2.13	V
G08	3S2	57	V	95	V	57	V	95	V	2.13	V	2.68	V	2.08	V
G08	3-3	67	V	111	V	67	V	111	V	2.50	V	3.13	V	2.44	V
G08	SU4	42	V	70	V	42	V	70	V	1.97	V	2.47	V	2.13	V
G08	SU5	43	V	73	V	43	V	73	V	1.83	V	2.29	V	1.97	V
G08	SU6	46	V	77	V	46	V	77	V	1.76	V	2.21	V	1.91	V
G08	SU7	49	V	82	V	49	V	82	V	1.72	V	2.17	V	1.87	V

Member Descr: 01-EG009

Mem ID	Truck	Rating (Tons)				Rating (Tons)				Rating Factor					
		ASD		LFD		ASD		LFD		LRFD		LRFD		LGL	
		INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	LGL	Contr Type
G09	HL93	79	V	132	V	79	V	132	V	1.84	V	2.33	V		
G09	HS20	79	V	132	V	79	V	132	V	2.16	V	2.72	V		
G09	3	75	M	125	M	75	M	125	M	2.81	V	3.53	V	2.73	V
G09	3S2	101	V	168	V	101	V	168	V	2.75	V	3.48	V	2.68	V
G09	3-3	117	V	196	V	117	V	196	V	3.23	V	4.07	V	3.15	V
G09	SU4	71	M	119	M	71	M	119	M	2.53	V	3.19	V	2.74	V
G09	SU5	75	M	125	M	75	M	125	M	2.34	V	2.95	V	2.53	V
G09	SU6	75	M	126	M	75	M	126	M	2.26	V	2.86	V	2.45	V
G09	SU7	77	M	129	M	77	M	129	M	2.21	V	2.81	V	2.40	V

Member Descr: 01-EG010

Mem ID	Truck	Rating (Tons)				Rating (Tons)				Rating Factor					
		ASD		LFD		ASD		LFD		LRFD		LRFD		LGL	
		INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	LGL	Contr Type
G10	HL93	78	V	131	V	78	V	131	V	1.84	V	2.33	V		
G10	HS20	78	V	131	V	78	V	131	V	2.15	V	2.72	V		
G10	3	75	M	124	M	75	M	124	M	2.80	V	3.53	V	2.73	V
G10	3S2	100	V	167	V	100	V	167	V	2.74	V	3.47	V	2.67	V
G10	3-3	116	V	194	V	116	V	194	V	3.22	V	4.07	V	3.14	V
G10	SU4	71	M	118	M	71	M	118	M	2.52	V	3.18	V	2.73	V
G10	SU5	74	M	124	M	74	M	124	M	2.33	V	2.94	V	2.53	V
G10	SU6	75	M	125	M	75	M	125	M	2.25	V	2.85	V	2.44	V
G10	SU7	77	M	129	M	77	M	129	M	2.21	V	2.80	V	2.40	V

Structure No.:	9999-999	Route:	I ###	Cycle No.:	17
Name:	I ### over Roadway			Insp. Date:	7/13/2012

Member Descr: 01-IG011

Mem ID	Truck	Rating (Tons)				Rating (Tons)				Rating Factor					
		ASD		LFD		LRFD		LRFD		LRFD		LRFD		LRFD	
		INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	LGL	Contr Type
G11	HL93	44	V	74	V	44	V	74	V	1.44	V	1.82	V		
G11	HS20	44	V	74	V	44	V	74	V	1.68	V	2.11	V		
G11	3	42	V	71	V	42	V	71	V	2.17	V	2.72	V	2.12	V
G11	3S2	56	V	94	V	56	V	94	V	2.13	V	2.68	V	2.08	V
G11	3-3	66	V	110	V	66	V	110	V	2.49	V	3.12	V	2.43	V
G11	SU4	42	V	70	V	42	V	70	V	1.97	V	2.46	V	2.12	V
G11	SU5	43	V	72	V	43	V	72	V	1.82	V	2.28	V	1.97	V
G11	SU6	45	V	76	V	45	V	76	V	1.76	V	2.21	V	1.90	V
G11	SU7	48	V	81	V	48	V	81	V	1.72	V	2.17	V	1.86	V

Member Descr: 01-IG012

Mem ID	Truck	Rating (Tons)				Rating (Tons)				Rating Factor					
		ASD		LFD		LRFD		LRFD		LRFD		LRFD		LRFD	
		INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	LGL	Contr Type
G12	HL93	42	V	71	V	42	V	71	V	1.19	V	1.54	V		
G12	HS20	42	V	71	V	42	V	71	V	1.45	V	1.88	V		
G12	3	41	V	68	V	41	V	68	V	2.00	V	2.60	V	1.95	V
G12	3S2	56	V	94	V	56	V	94	V	1.66	V	2.16	V	1.62	V
G12	3-3	63	V	105	V	63	V	105	V	1.94	V	2.51	V	1.89	V
G12	SU4	40	V	66	V	40	V	66	V	1.82	V	2.36	V	1.99	V
G12	SU5	41	V	69	V	41	V	69	V	1.63	V	2.12	V	1.79	V
G12	SU6	43	V	73	V	43	V	73	V	1.54	V	2.00	V	1.69	V
G12	SU7	46	V	77	V	46	V	77	V	1.47	V	1.90	V	1.61	V

Member Descr: 01-IG013

Mem ID	Truck	Rating (Tons)				Rating (Tons)				Rating Factor					
		ASD		LFD		LRFD		LRFD		LRFD		LRFD		LRFD	
		INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	LGL	Contr Type
G13	HL93	43	V	72	V	43	V	72	V	1.20	V	1.55	V		
G13	HS20	43	V	72	V	43	V	72	V	1.46	V	1.89	V		
G13	3	41	V	69	V	41	V	69	V	2.01	V	2.61	V	1.96	V
G13	3S2	56	V	94	V	56	V	94	V	1.67	V	2.16	V	1.62	V
G13	3-3	64	V	107	V	64	V	107	V	1.95	V	2.52	V	1.89	V
G13	SU4	40	V	67	V	40	V	67	V	1.82	V	2.36	V	1.99	V
G13	SU5	42	V	70	V	42	V	70	V	1.64	V	2.12	V	1.79	V
G13	SU6	44	V	74	V	44	V	74	V	1.55	V	2.01	V	1.69	V
G13	SU7	47	V	78	V	47	V	78	V	1.47	V	1.91	V	1.61	V

Member Descr: 01-IG014

Mem ID	Truck	Rating (Tons)				Rating (Tons)				Rating Factor					
		ASD		LFD		LRFD		LRFD		LRFD		LRFD		LRFD	
		INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	LGL	Contr Type
G14	HL93	43	V	72	V	43	V	72	V	1.20	V	1.55	V		
G14	HS20	43	V	72	V	43	V	72	V	1.46	V	1.89	V		
G14	3	41	V	69	V	41	V	69	V	2.01	V	2.61	V	1.96	V
G14	3S2	56	V	94	V	56	V	94	V	1.67	V	2.16	V	1.62	V

Structure No.:	9999-999	Route:	I ###	Cycle No.:	17
Name:	I ### over Roadway			Insp. Date:	7/13/2012

G14	3-3	64	V	107	V	64	V	107	V	1.95	V	2.52	V	1.89	V
G14	SU4	40	V	67	V	40	V	67	V	1.82	V	2.36	V	1.99	V
G14	SU5	42	V	70	V	42	V	70	V	1.64	V	2.12	V	1.79	V
G14	SU6	44	V	74	V	44	V	74	V	1.55	V	2.01	V	1.69	V
G14	SU7	47	V	78	V	47	V	78	V	1.47	V	1.91	V	1.61	V

Member Descr: 01-IG015

Mem ID	Truck	Rating (Tons)				Rating (Tons)				Rating Factor					
		ASD		LFD		LRFD		LRFD		LRFD		LRFD		LRFD	
		INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	LGL	Contr Type
G15	HL93	43	V	72	V	43	V	72	V	1.20	V	1.55	V		
G15	HS20	43	V	72	V	43	V	72	V	1.46	V	1.89	V		
G15	3	41	V	69	V	41	V	69	V	2.01	V	2.61	V	1.96	V
G15	3S2	56	V	94	V	56	V	94	V	1.67	V	2.16	V	1.62	V
G15	3-3	64	V	107	V	64	V	107	V	1.95	V	2.52	V	1.89	V
G15	SU4	40	V	67	V	40	V	67	V	1.82	V	2.36	V	1.99	V
G15	SU5	42	V	70	V	42	V	70	V	1.64	V	2.12	V	1.79	V
G15	SU6	44	V	74	V	44	V	74	V	1.55	V	2.01	V	1.69	V
G15	SU7	47	V	78	V	47	V	78	V	1.47	V	1.91	V	1.61	V

Member Descr: 01-IG016

Mem ID	Truck	Rating (Tons)				Rating (Tons)				Rating Factor					
		ASD		LFD		LRFD		LRFD		LRFD		LRFD		LRFD	
		INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	LGL	Contr Type
G16	HL93	43	V	72	V	43	V	72	V	1.20	V	1.55	V		
G16	HS20	43	V	72	V	43	V	72	V	1.46	V	1.89	V		
G16	3	41	V	69	V	41	V	69	V	2.01	V	2.61	V	1.96	V
G16	3S2	56	V	94	V	56	V	94	V	1.67	V	2.16	V	1.62	V
G16	3-3	64	V	107	V	64	V	107	V	1.95	V	2.52	V	1.89	V
G16	SU4	40	V	67	V	40	V	67	V	1.82	V	2.36	V	1.99	V
G16	SU5	42	V	70	V	42	V	70	V	1.64	V	2.12	V	1.79	V
G16	SU6	44	V	74	V	44	V	74	V	1.55	V	2.01	V	1.69	V
G16	SU7	47	V	78	V	47	V	78	V	1.47	V	1.91	V	1.61	V

Member Descr: 01-IG017

Mem ID	Truck	Rating (Tons)				Rating (Tons)				Rating Factor					
		ASD		LFD		LRFD		LRFD		LRFD		LRFD		LRFD	
		INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	LGL	Contr Type
G17	HL93	43	V	72	V	43	V	72	V	1.20	V	1.55	V		
G17	HS20	43	V	72	V	43	V	72	V	1.46	V	1.89	V		
G17	3	41	V	69	V	41	V	69	V	2.01	V	2.61	V	1.96	V
G17	3S2	56	V	94	V	56	V	94	V	1.67	V	2.16	V	1.62	V
G17	3-3	64	V	107	V	64	V	107	V	1.95	V	2.52	V	1.89	V
G17	SU4	40	V	67	V	40	V	67	V	1.82	V	2.36	V	1.99	V
G17	SU5	42	V	70	V	42	V	70	V	1.64	V	2.12	V	1.79	V
G17	SU6	44	V	74	V	44	V	74	V	1.55	V	2.01	V	1.69	V
G17	SU7	47	V	78	V	47	V	78	V	1.47	V	1.91	V	1.61	V

Member Descr: 01-EG018

Rating (Tons)	Rating (Tons)	Rating Factor
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Structure No.: 9999-999 Route: I ### Cycle No.: 17
 Name: I ### over Roadway Insp. Date: 7/13/2012

Mem ID	Truck	ASD				LFD				LRFD					
		INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	INV	Contr Type	OPR	Contr Type	LGL	Contr Type
G18	HL93	87	V	146	V	87	V	146	V	1.98	V	2.57	V		
G18	HS20	87	V	146	V	87	V	146	V	2.41	V	3.13	V		
G18	3	84	V	140	V	84	V	140	V	3.33	V	4.32	V	3.24	V
G18	3S2	113	V	188	V	113	V	188	V	2.77	V	3.59	V	2.69	V
G18	3-3	130	V	217	V	130	V	217	V	3.22	V	4.18	V	3.13	V
G18	SU4	82	V	137	V	82	V	137	V	3.02	V	3.92	V	3.30	V
G18	SU5	85	V	141	V	85	V	141	V	2.72	V	3.52	V	2.97	V
G18	SU6	90	V	150	V	90	V	150	V	2.56	V	3.32	V	2.81	V
G18	SU7	95	V	159	V	95	V	159	V	2.44	V	3.16	V	2.67	V

Structure No.: 9999-999 Route: I ### Cycle No.: 17
 Name: I ### over Roadway Insp. Date: 7/13/2012

CALCULATIONS - LARS DATA ECHO REPORT:

LOAD ANALYSIS AND RATING INPUT v5.00.06.09

 * Form Type 01 - Batch Specifications *

Date: 10/26/11

Rating Analyst:JAS

	Inv.	Oper	Post
Rating Type Exceptions:	A	A	A

Inventory Load Name: HL93

Operating Load Name: HL93 Allowable Stress Ratio / Inv.:

Posting Load Name: 3 Allowable Stress Ratio / Inv.:

Posting Load Name: 3S2

Posting Load Name: 3-3

Special Allowable Stress Ratio / Inv.:

"POST" always: POST

 * Form Type 02 - Structure Header and Description *

Structure I.D.: 999999

Rating Analyst:JAS

Engineer's Attention:

	Inv.	Oper	Post
Rating Type Exceptions:			

Floor beam single lane:

Curb distance: inches

Structure Type: CPS

Year of Construction: 1963

Structure Length: 62.000 feet

Roadway Width: 55.50 feet

Number of Spans: 1

Negate Special Load Analysis:

Inventory Load Name:

Operating Load Name:

 * Form Type 03 - Non-Standard Live Load Truck Description *

Structure No.: 9999-999 Route: I ### Cycle No.: 17
 Name: I ### over Roadway Insp. Date: 7/13/2012

Load Name: 3

Number of Axle Loads: 3

Seq.	Axle 1		Axle 2		Axle 3		Axle 4	
	Load	Spacing	Load	Spacing	Load	Spacing	Load	Spacing
1	16.0	15.000	17.0	4.000	17.0	0.000		0.000

Load Name: 3-3

Number of Axle Loads: 6

Seq.	Axle 1		Axle 2		Axle 3		Axle 4	
	Load	Spacing	Load	Spacing	Load	Spacing	Load	Spacing
1	12.0	15.000	12.0	4.000	12.0	15.000	16.0	16.000

Seq.	Axle 5		Axle 6		Axle 7		Axle 8	
	Load	Spacing	Load	Spacing	Load	Spacing	Load	Spacing
2	14.0	4.000	14.0	0.000		0.000		0.000

Load Name: 3S2

Number of Axle Loads: 5

Seq.	Axle 1		Axle 2		Axle 3		Axle 4	
	Load	Spacing	Load	Spacing	Load	Spacing	Load	Spacing
1	12.0	11.000	17.0	4.000	17.0	22.000	17.0	4.000

Seq.	Axle 5		Axle 6		Axle 7		Axle 8	
	Load	Spacing	Load	Spacing	Load	Spacing	Load	Spacing
2	17.0	0.000		0.000		0.000		0.000

Load Name: HL93

Number of Axle Loads: 3

Seq.	Axle 1		Axle 2		Axle 3		Axle 4	
	Load	Spacing	Load	Spacing	Load	Spacing	Load	Spacing
1	8.00	14.000	32.0	14.000	32.0	0.000		0.000

 * Form Type 04 - LRFD System Factors *

Structure I.D.: 999999

System Factor: 0.000

ADTT: 8050

 * Form Type 05 - Structure Location and Permanent Identification Factors *

Structure No.: 9999-999 Route: I ### Cycle No.: 17
 Name: I ### over Roadway Insp. Date: 7/13/2012

Structure I.D.: 999999

Bridge Number: I ### ov

District: 1

County: Mor

Construction Route:
 Section:
 Station: + .

Microfilm Reel Number Design Plans:
 Computations:
 Correspondence:

Key Route I.D.:

Marked Route:

 * Form Type 06 - Comments *

Structure I.D.: 999999

NBI ID 9999999

Facility Carried I ###

Feature Intersected Roadway

 * Form Type 07 - Material Strength Input *

Structure I.D.: 999999

LOSS: Eg/Es: 1.300 f's: f''c:

	Inv.	Oper	Post	Spec
Maximum Impact Factor:	30%	30%	30%	30%

	Inv.	Oper	Post	Spec
Minimum Impact Factor:	10%	10%	10%	10%

 * Form Type 44 - LRFD Member Factors *

System Factor: 0.000

Condition Factor: 0.000

Structure No.: 9999-999 Route: I ### Cycle No.: 17
 Name: I ### over Roadway Insp. Date: 7/13/2012

* Form Type 08 - Member Description Specifications *

Structure I.D.: 999999

Member I.D.: G12

Number of Spans: 1 Symmetry:

	Span 1	Span 2	Span 3	Span 4	Span 5	Span 6
Span Lengths:	56.79	0.00	0.00	0.00	0.00	0.00

Material Type: CPS

f's: 250 f''c: 3000

Live Load Distribution Factor: 1.21

Shear Live Load Dist. Factor: 1.5

	Inv.	Oper	Post	Spec
Maximum Impact Factor:	30%	30%	30%	30%

Stiffener Code:

 * Form Type 48 - LRFD Live Load Distribution Factor *

Structure I.D.: 999999

Member I.D.: G12

		Span 1				Span 2				Span 3		
		R1	R2	R3		R1	R2	R3		R1	R2	R3
1-lane DF		0.495	0.000	0.000		0.000	0.000	0.000		0.000	0.000	0.000
1-lane ft		56.786	0.000	0.000		0.000	0.000	0.000		0.000	0.000	0.000

		Span 1				Span 2				Span 3		
		R1	R2	R3		R1	R2	R3		R1	R2	R3
2-lane DF		0.662	0.000	0.000		0.000	0.000	0.000		0.000	0.000	0.000
2-lane ft		56.786	0.000	0.000		0.000	0.000	0.000		0.000	0.000	0.000

LRFD LLDF Shear

		Span 1				Span 2				Span 3		
		R1	R2	R3		R1	R2	R3		R1	R2	R3
1-lane DF		0.643	0.000	0.000		0.000	0.000	0.000		0.000	0.000	0.000
1-lane ft		56.786	0.000	0.000		0.000	0.000	0.000		0.000	0.000	0.000

		Span 1				Span 2				Span 3		
		R1	R2	R3		R1	R2	R3		R1	R2	R3
2-lane DF		0.738	0.000	0.000		0.000	0.000	0.000		0.000	0.000	0.000
2-lane ft		56.786	0.000	0.000		0.000	0.000	0.000		0.000	0.000	0.000

 * Form Type 40 - Factors *

Structure I.D.: 999999

Member I.D.: G12

LRFD Impact Factor: 0.330

Structure No.: 9999-999 Route: I ### Cycle No.: 17
 Name: I ### over Roadway Insp. Date: 7/13/2012

AASHTO Multiplier - ASD/LFD Impact: 1.000

 * Form Type 10 - Superimposed Dead Loads *

Structure I.D.: 999999

Member I.D.: G12

Symmetry:

Span No.	Load Type	Dist. from Left Supp.	Distributed Load (lbs/ft)		Concentrated Load (kips)
			Left	Right	Length
1	W	0.000	217.7	217.7	56.786
1	Z	0.000	209.9	209.9	56.786

 * Form Type 11 - Section Range Specifications *

Structure I.D.: 999999

Member I.D.: G12

Symmetry:

Range Length -- Non-Composite:

Span No.	Range No.	Range Length	Sect. Left	Sect. Right	Sect. Var.	Hinge Location No. 1	Hinge Location No. 2	Hybrid Yield Base Fy	Yield Exception Fy
1	1	56.786	1			0.0	0.0		

 * Form Type 15 - Prestressed Concrete Properties *

Structure I.D.: 999999

Member I.D.: G12

Section Number	Void Depth or Diam.	Void Width	Void dy
1			
1			
1			
1			
1			
1			

Section Number	Element	Prestressing Strands			
		Code	As	Code	Dy
1	1	L	1004		2.000

Structure No.: 9999-999 Route: I ### Cycle No.: 17
 Name: I ### over Roadway Insp. Date: 7/13/2012

1	2	L	1004	4.000
1	3	L	1004	6.000
1	4	L	804	8.000
1	5	L	604	10.00
1	6	L	404	12.00

Sect. No.	Same/ Except	Code	Beam Depth Width	Top Flg. Thk.	Top Flg. Thk.	Top Fil.	Web Thk.	Bott. Fil. Thk.	Bott. Flg. Width	Bott. Flg.
1			45.00	16.0	07.0	04.50	07.0	07.50	07.0	22.0
1			00.00	00.0	00.0	00.00	00.0	00.00	00.0	00.0
1			00.00	00.0	00.0	00.00	00.0	00.00	00.0	00.0
1			00.00	00.0	00.0	00.00	00.0	00.00	00.0	00.0
1			00.00	00.0	00.0	00.00	00.0	00.00	00.0	00.0
1			00.00	00.0	00.0	00.00	00.0	00.00	00.0	00.0

 * Form Type 14 - Composite Concrete Properties *

Structure I.D.: 999999

Member I.D.: G12

Composite Slab Properties:

Sym.	Span No.	Range Number	Range Length	Comp. Code	n value
	1	1	56.8	C	

Composite Section Description:

Sect. No.	Same/ Except	Code	Slab Width	Slab Thk.	Fillet Width	Fillet Thk.	Eff. Width	Eff. Thk.	Dy to top of Sect.
1			80.00	7.50	16.0	2.25	80.00	7.50	6.000

 * Form Type 17 - Shear Reinforcement Specifications *

Structure I.D.: 999999

Member I.D.: G12

Symmetry:

Span No.	Range No.	S B	Range Length	Group Code	Angle of reinf.	Shear i	Reinforcement Code	As	No. of Spaces	Equally Spaced Spaces in Range
1	1	S	0.250			01	B	204	1	0.250
1	2	S	1.000			01	B	204	3	0.333
1	3	S	0.750			01	B	204	1	0.750
1	4	S	6.964			01	B	204	13	0.536
1	5	S	9.464			01	B	204	9	1.051
1	6	S	9.465			01	B	204	6	1.577
1	7	S	1.000			01	B	204	1	1.000

Structure No.:	<u>9999-999</u>			Route:	<u>I ###</u>			Cycle No.:	<u>17</u>
Name:	<u>I ### over Roadway</u>							Insp. Date:	<u>7/13/2012</u>

1	8	S	9.465	01	B	204	6	1.577
1	9	S	9.464	01	B	204	9	1.051
1	10	S	6.964	01	B	204	13	0.536
1	11	S	0.750	01	B	204	1	0.750
1	12	S	1.000	01	B	204	3	0.333
1	13	S	0.250	01	B	204	1	0.250